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June 14, 2021

Mr. Eddie Barry
ConnectGen LLC
1001 McKinney Street, Suite 700
Houston, TX 77002

Re: Geotechnical Engineering Report
Mill Point Solar Project
Town of Glen
Montgomery County, New York
TRC Project No.: 411360.GEO1

Dear Mr. Barry:

TRC Engineers, Inc. (TRC) is pleased to present our Geotechnical Engineering Report for this project. Our work was initiated in accordance with your authorization to proceed (Task Order # 03) dated March 4, 2021 and completed in general accordance with our agreed scope of work presented in our revised proposal, submitted February 10, 2021. A summary of our geotechnical investigation activities, findings and recommendations is summarized below.

1.0 INTRODUCTION

This report presents the results of our geotechnical investigation for the proposed 250 MW photovoltaic (PV) solar array to be constructed at the Mill Point project site. The Mill Point project consists of multiple parcels on the order of 3,000 acres , located South of Interstate 90 in the Town of Glen, New York. The purpose of our investigation was to evaluate the geologic and subsurface conditions to reduce uncertainty with respect to anticipated foundation and site construction, and to provide preliminary geotechnical recommendations for design of the proposed project.

1.1 Project Description

The site is located South of Interstate 90 in the Town of Glen, in Montgomery County, NY. The project includes PV solar arrays, ancillary equipment, and proposed substation facilities on the north side of Ingersoll Road. Based on our experience with similar projects, we assume that the proposed photovoltaic arrays would preferably be mounted on posts driven into the ground. The anticipated post loads have not been provided but are assumed to be typical for such construction. It is assumed that existing grades will remain relatively unchanged.

1.2 Scope of Services

Our scope of services was presented in our Proposal for Geotechnical Engineering Services dated February 10, 2021. To accomplish this work, we have provided the following services:

- Exploration of subsurface conditions by drilling thirty (30) borings spread across the proposed solar array areas, drilling two (2) additional test borings within the proposed substation areas, and retrieving soil samples for classification & laboratory testing.

- Evaluation of the physical and engineering properties of the subsurface soils based on visually classifying the samples by a member of our geotechnical staff.

- Engineering analysis to evaluate the proposed foundation systems for the support of the ground-mounted PV solar arrays and associated equipment.

- Preparation of this report to summarize our findings and to present our conclusions and recommendations regarding the following:

    - Foundation support for the proposed solar array structures assuming post foundations, or alternative system as applicable based on subsurface conditions.
    - Preliminary bearing capacity and other parameters for use in preliminary foundation design.
    - Anticipated excavation conditions and presence of potential rock or other refusal conditions, if applicable.
    - Suitability of on-site soils for reuse in backfills and requirements for imported fills.
    - Recommendations for placement, compaction, and testing of fills, if applicable.
    - Preliminary soil parameters pressures (both above and below ground water table) for active, at rest, and passive conditions and L-Pile soil parameters for use in foundation design
    - Anticipated ground water conditions and impacts on the design and construction.
    - Frost penetration depth.
    - Corrosivity concerns on buried steel and concrete.
    - Thermal resistivity results.
    - Preliminary Seismic Site Class parameters.
    - Other construction-related concerns, as warranted based on site subsurface conditions, details of the proposed construction, and any available preliminary design information.

2.0 SITE CONDITIONS

2.1 Site Reconnaissance, Boring Stakeout and Investigation

TRC's field staff performed a site reconnaissance in conjunction with test boring stakeout. Test boring locations were staked in the field using a hand-held GPS unit at the approximate locations recommended by TRC's geotechnical staff and approved by the Client as shown on the Test Boring Location Plan. The site is mostly open agricultural fields covered by seasonal crops along the array field. Some wooded areas are present in many of the proposed parcel areas. Prior to drilling, the Dig Safely NY notification system was contacted to check the presence of public utilities in the area of the proposed testing borings.

The test boring work was performed during the period from April 27, 2021 to May 4, 2021 by TRC's drilling subcontractor, CME Associates. Drilling and sampling were performed using an ATV-mounted drill rig in general accordance with ASTM D 1586. Split spoon sampling was performed continuously through the upper 10 ft and at 5 ft intervals thereafter to the completion depths of each boring, unless refusal to drilling tools was encountered prior to these depths. Borings were terminated at depths ranging from 6.5 to 15 ft below existing ground surface (bgs) within the proposed solar array areas. Test borings for the proposed substation locations were extended to depths of 35 ft bgs each. Upon completion, all test borings were backfilled to the approximate existing ground surface with the auger cuttings. Copies of the test boring logs and a Test Boring Location plan identifying approximate borings locations are attached.

2.2 Geology

According to available geological data, the surficial geology at the project site consists of residual soils. Locally the site is underlain predominantly by mudstone and shale of the Canajoharie Shale Formation from the Middle Ordovician Age. The northern parcels sit near a contact with carbonate limestone of the Trenton and Black River Groups, as well as Quarternary Age glacial and alluvial deposits.

2.3 Subsurface Conditions

The test borings revealed that the project site is generally covered with a surficial layer of topsoil approximately 3 inches thick. Below the surficial topsoil, the subsurface conditions consisted of brown to dark brown clays and silts with varying quantities of sand and gravel or gravel-sized rock fragments. Standard Penetration (SPT) N-values indicate that the consistency of this layer ranges from "medium" and "stiff" in the upper 2 to 4 ft bgs and generally increasing to "very stiff" to "hard with depth. Laboratory test results performed on representative samples indicate plastic limits ranging from approximately 13% to 28%, liquid limits ranging from 18% to 52% and plasticity indices ranging from 4% to 25%.

Natural moisture contents range from approximately 10% to 40% and dry unit weights ranged from approximately 93.9 to 124.8 pounds per cubic foot (pcf). Maximum laboratory compacted dry density of five composite bulk samples as determined by ASTM D 698 ranged from 96.2 to 119 pcf at optimum moisture contents ranging from 11.8% to 18.4%.

Occasional strata of cobbles and boulders were encountered in various borings ranging from the depth of 3 ft to 15 ft. The SPT N-values for these strata indicate the consistency of very dense to refusal. The presence of this strata may pose difficult driving conditions for driven post type foundation during installation.

Auger refusal, which typically represents the presence of weathered rock was encountered in ten (10) of the borings, sporadically located throughout the exploration area. Refusal depths varied between 6.5 ft and 9.5 ft, dipping away from Mohawk River. Difficult drilling conditions which are typically indicative of very dense till type of soil and/or decomposed rock were also encountered in 21 of the 32 test boring locations. The depths and locations where difficult drilling and auger refusal were encountered are summarized in Table 1, below.

Table 1. Summary of Difficult Drilling and Auger Refusal Depths

Test Boring Location	Depth to Very Dense Soils/Difficult Drilling (ft)	Depth to Auger Refusal (ft)
B-01	6.5	6.9
B-02	8	>15
B-03	8	>15
B-04	7	>15
B-05	7	>15
B-06	7	7.7
B-07	8	>15
B-08	>15	>15
B-09	10	13.9
B-10	7.8	>15
B-11	6	7.6
B-12	5	>15
B-13	>15	>15
B-14	13.5	>15
B-15	9	>15
B-16	>15	>15
B-17	9	10.5
B-18	6	8
B-19	6	7.5

Test Boring Location	Depth to Very Dense Soils/Difficult Drilling (ft)	Depth to Auger Refusal (ft)
B-20	>15	>15
B-21	>15	>15
B-22	>15	>15
B-23	6	>15
B-24	8	>15
B-25	>15	>15
B-26	>15	>15
B-27	13.5	>15
B-28	8	>15
B-29	7	14.6
B-30	5.5	6.5
SS-01	7	>35
SS-02	9	>35

2.4 Ground Water

Groundwater was encountered during drilling at the time of the field investigation in seven (7) of the test boring locations as summarized in Table 2 below:

Table 2. Summary of Groundwater Conditions

Test Boring Location	Groundwater Depth (ft)
B-08	12.5
B-09	10.3
B-24	10.3
B-25	14.2
B-28*	3
B-30*	2.5
SS-02	8.2

* *Possible perched water*

Groundwater and/or the development of perched water conditions may be encountered within standard excavation depths for foundations or utilities during wet periods. The groundwater conditions are representative of the conditions at the date and time of this study and are not representative of daily, seasonal, long term fluctuations, development of perched conditions, or ponding of water in low lying areas during wet periods.

3.0 CORROSION EVALUATION AND THERMAL RESTIVITY

3.1 Corrosion Evaluation

To evaluate the corrosion potential of the subsurface soils at the site, we submitted five (5) composite bulk soil samples collected from test boring locations (approximately 0-5 ft bgs) during our subsurface investigation to an analytical laboratory for pH, resistivity, soluble sulfate and chloride content testing. The results are summarized in Table 3, below.

Table 3. Results of Corrosivity Testing

Sample	Boring No.	Chloride (mg/kg)*	Sulfate (mg/kg)*	pH	Resistivity (ohm-cm)**	Estimated Corrosivity Based on Resistivity	Estimated Corrosivity Based on Sulfates
Bulk 1	B-1 to B-5	50	58	7.7	2,548	Moderately Corrosive	Negligible
Bulk 2	B-6 to B-8	40	55	8.1	2,940	Moderately Corrosive	Negligible
Bulk 3	B-9 to B-14	40	220	8.4	1,260	Severely Corrosive	Negligible
Bulk 4	B-22 to B-29	38	235	8.2	1,176	Severely Corrosive	Negligible
Bulk 5	SS-1 & SS-2	75	185	8.3	1,568	Severely Corrosive	Negligible

* *mg/kg = milligrams per kilogram*
** *ohm-cm = ohm-centimeter*

TRC also conducted ten (10) field resistivity testing using the Wenner Four-Pin method in general accordance with ASTM G57. Tests were centered at boring locations B-1, B-4, B-9, B-12, B-13, B-16, B-20, B-22, B-27, and SS-2 with the test lines oriented perpendicular to one another at each test location. Measurements were taken along each test line corresponding to electrode spacings of 2.5 ft, 5 ft, 10 ft, 20 ft, and 40 ft. Field resistivity test results are attached, and the results are discussed further in this section.

Many factors can affect the corrosion potential of soil including soil moisture content, resistivity, permeability and pH, as well as chloride and sulfate concentration. In general, soil resistivity, which is a measure of how easily electrical current flows through soils, is the most influential factor. Based on classification developed by William J. Ellis (1978), the approximate relationship between soil corrosiveness was developed as shown in Table 4 below.

Table 4. Relationship Between Soil Resistivity and Soil Corrosivity

Soil Resistivity (ohm-cm)*	Classification of Soil Corrosiveness
0 to 900	Very Severely Corrosive
900 to 2,300	Severely Corrosive
2,300 to 5,000	Moderately Corrosive
5,000 to 10,000	Mildly Corrosive
10,000 to >100,000	Very Mildly Corrosive

** ohm-cm = ohm-centimeter*

Chloride and sulfate ion concentrations and pH appear to play secondary roles in affecting corrosion potential. High chloride levels tend to reduce soil resistivity and break down otherwise protective surface deposits, which can result in corrosion of buried metallic improvements or reinforced concrete structures. Sulfate ions in the soil can lower the soil resistivity and can be highly aggressive to Portland cement concrete (PCC) by combining chemically with certain constituents of the concrete, principally tricalcium aluminate. This reaction is accompanied by expansion and eventual disruption of the concrete matrix. Soils containing high sulfate content could also cause corrosion of the reinforcing steel in concrete. Table 4.2.1 of the American Concrete Institute (ACI, 2008) provides requirements for concrete exposed to sulfate-containing solutions as summarized in Table 5.

Table 5. Relationship Between Sulfate Concentration and Sulfate Exposure (Table 4.2.1 of ACI)

Water-Soluble Sulfate (SO4) in soil (ppm)*	Sulfate Exposure
0 to 1,000	Negligible
1,000 to 2,000	Moderate
2,000 to 20,000	Severe
over 20,000	Very Severe

* *ppm = parts per million*

Acidity is an important factor of soil corrosivity. The lower the pH (the more acidic the environment), the higher will the soil corrosivity be with respect to buried metallic structures. As soil pH increases above 7 (the neutral value), the soil is increasingly more alkaline and less corrosive to buried steel structures due to protective surface films which form on steel in high pH environments. A pH between 5 and 8.5 is generally considered relatively passive from a corrosion standpoint.

The laboratory electrical resistivity tests completed on the composite samples of surficial soils indicate values ranging from 1,176 to 2,940 ohm-centimeters, which would be indicative of moderately to severely corrosive potential to buried metallic improvements. Based on the field resistivity testing results, the electrical resistivity

values for the existing subsoils range from approximately 3,275 to 69,132 ohm-centimeters. Based on these results and the resistivity correlations presented in Table 4, the corrosion potential to buried metallic improvements may be characterized a ranging from moderately corrosive to severely corrosive.

Based on our previous experience and Table 4.2.1 of the ACI, it is our opinion that sulfate exposure to PCC may be considered negligible for the native subsurface materials sampled.

3.2 Thermal Resistivity

The thermal resistivity test results with the thermal dryout curves, are attached to this report. Thermal Resistivity testing was performed in general accordance with ASTM 5334 on five composite samples compacted to density equivalent to approximately 90% of the maximum dry density per ASTM D 698 and at the optimum moisture contents for each composite test sample. The samples were then oven dried and multiple thermal resistivity readings were obtained at various moisture contents. The thermal resistivities decrease with increasing moisture content and varies from 100 to 565 ^{o}C-cm/W when fully dry and from 46.8 to 111.5 ^{o}C-cm/W at optimum moisture.

4.0 FOUNDATIONS AND EARTHWORK

4.1 Site Seismic Coefficients

According to the 2018 International Building Code, the site class is within “Site Class C” based on the soil profiles. The maximum considered earthquake ground motions in this area for 0.2 sec. and 1.0 sec. spectral responses are approximately 21.4 % g and 6.2 % g, respectively. For Site Class C, the corresponding 0.2 and 1.0 sec. design spectral response acceleration parameters S_{DS} and S_{D1} are 18.5 % g and 6.2 % g, respectively.

4.2 Foundations

Based on the results of this investigation and our experience with similar structures, a foundation system consisting of driven posts is generally preferred for support of the proposed ground-mounted photovoltaic arrays. Boring locations were divided in five (5) different zone for engineering characterization purposes. Based on the results of the test borings, the use of driven posts could be problematic in zones 2 and 3 (Table 5) due to very dense soil and shallow refusal conditions

As noted in Table 1, nine (9) test borings encountered refusal to earth drilling equipment at depths ranging from 6.5 to nearly 15 feet bgs. Additionally, difficult drilling conditions and/or very dense soil conditions including cobble layers were encountered in twenty (20) of the thirty two (32) test borings at depths ranging from 5 ft to 10 ft ft bgs. Therefore,

shallow refusal conditions may be encountered within these areas and other portions of the proposed solar array areas when attempting to drive posts.

Since the use of a driven post system may be limited for use on this project where refusal to drilling and sampling tools is encountered, the designer and contractor should be prepared to implement alternative installation methods (or alternative foundation support systems) for achieving sufficient foundation embedment to provide sufficient resistance for uplift and lateral loading condition, as necessary. The following alternatives will need to be considered at the project site since subsurface obstructions due to likely highly decomposed rock or possible cobbles are anticipated at depths less than 10 ft at six (6) out of thirty (32) test boring locations in addition to possible difficult driving conditions due to very dense residual soils:

- The use of predrilling or spudding with a heavy steel beam to break up the dense highly decomposed rock or other obstructions to increase post embedment for vertical and lateral support.
- The use of larger sized, heavier grade posts that will allow harder driving and could provide increased embedment and sufficient lateral capacity and uplift.
- The use of helical screw piles to achieve uplift and lateral capacities at shallower depths.
- The use of shallow spread footings or ballast foundations where adequate embedment with other foundation or installation methods cannot be achieved.

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4.2.1 Driven Post Support System

As mentioned above, driving post beyond depths where very dense soils, cobble layers, and highly decomposed rock were encountered will be difficult and pre-drilling will likely become necessary to achieve sufficient post depth to resist the required lateral and uplift loads wherever similar conditions are encountered. All posts should be driven to bear at sufficient depths required to provide adequate axial, uplift, and lateral resistances.

4.2.2 Helical Screw Support System

A helical pile system, such as that manufactured by IDEAL Manufacturing, AB Chance, Magnum Piering, or similar, having a minimum 3-inch diameter or low-displacement ground screws, such as those manufactured by TerraSmart, or similar, could be considered as an alternative to driven posts in areas where overburden depths are less than 8 ft for support of the proposed arrays. Lateral and uplift capacities of helical piles, as well as the ability of the shaft to withstand anticipated installation torque based on subsurface conditions, should be verified by the pile manufacturer or installer. Generally speaking, additional capacities can be developed using larger diameters and helix combinations. Installation of

helical piles below the auger refusal depths, where encountered, will not be feasible. Embedment into the very dense/difficult augering material may be possible, but as stated previously, will be dependent on the ability of the central shaft to withstand installation torque required to advance helices. Depths of very dense soils and auger refusal are as presented in Table 1 above and piles will not be able to penetrate below these depths Alternative to a conventional small shaft diameter helical pile, the use of a continuous flight helical pile, could be considered that generally can be drilled deeper into very dense soil conditions as compared to a conventional helical pile with larger diameter helices.

The final design should be verified by the helical or drilled pile manufacturer prior to implementation at the site. Also, the type and diameter of helix plates to be used, as well as the central bar or round pipe characteristics or that of a continuous flight helical pile should be verified by the product manufacturer based on this design capacity and anticipated torque value required for installation of the helical piles. If subsurface obstructions are encountered during installation, pre-drilling or pre-excavation will be required. If predrilling or pre-excavating, then all piles should be grouted to ensure intimate contact with surrounding soils and so not to negatively impact lateral stability.

Recommended geotechnical parameters for use in design analysis, included in Tables 6a through 6e below, can be utilized for evaluation of posts or piles for support of the PV solar array, or other design analysis, as required. We recommend that lateral and uplift resistance of soils be reduced by 50% above a depth of 4 ft below the ground surface to account for disturbance resulting from construction as well as to account for the negative impacts and loss of support due to frost and thaw action. A minimum factor of safety of 2 is recommended for compression loads; a factor of safety equal to 3 should be used for determining allowable uplift capacity of piles; a factor of safety equal to 1.5 should be used for transient (wind/seismic) loading conditions.

Table 6a. Summary of Unfactored Soil Parameters for Design
Zone 1: Borings B-1 through B-5
(reduce by 50% for upper 4 ft)

Soil Description	LPILE Soil Type	Relative Density / Consistency	Total Unit Weight (pcf*)	Friction Angle (degrees)	E_{50}	Cohesion (psf**)	Allowable Bearing Capacity (ksf***)
SILT & CLAY (0-4 ft)	Clay	"Medium" to "Stiff"	120	-	0.01	1,500	2
SILT & CLAY (4 ft+)	Clay	"Very Stiff" to "Hard"	125	-	0.005	3,000	4

* *pcf – pounds per cubic foot*
** *psf – pounds per square foot*
*** *ksf – kips per square foot*

Table 6b. Summary of Unfactored Soil Parameters for Design
Zone 2: Borings B-6 through B-8
(reduce by 50% for upper 4 ft)

Soil Description	LPILE Soil Type	Relative Density / Consistency	Total Unit Weight (pcf*)	Friction Angle (degrees)	E_{50}	Cohesion (psf**)	Soil Modulus, k (pci***)	Allowable Bearing Capacity (ksf****)
Silty CLAY (0-6 ft)	Clay	"Medium" to "Stiff"	115	-	0.01	1,500		1.5
Silty SAND & Silty Gravel (6 ft+)	Sand	"Medium Dense/Very Stiff" to "Dense/Hard"	125	34	-	-	225	4

* *pcf – pounds per cubic foot*
** *psf – pounds per square foot*
*** *pci – pounds per cubic inch*
**** *ksf – kips per square foot*

Table 6c. Summary of Unfactored Soil Parameters for Design
Zone 3: Borings B-9 through B-16
(reduce by 50% for upper 4 ft)

Soil Description	LPILE Soil Type	Relative Density / Consistency	Total Unit Weight (pcf*)	Friction Angle (degrees)	E_{50}	Cohesion (psf**)	Soil Modulus, k (pci***)	Allowable Bearing Capacity (ksf****)
Sandy and Clayey SILT (0-6 ft)	Clay	"Medium" to "Stiff"	115	-	0.01	1,500		2
SILT and Sandy SILT (6 ft+)	Sand	"Medium Dense/Very Stiff" to "Dense/Hard"	130	32	-	-	225	3

* *pcf – pounds per cubic foot*
** *psf – pounds per square foot*
*** *pci – pounds per cubic inch*
**** *ksf – kips per square foot*

Table 6d. Summary of Unfactored Soil Parameters for Design
Zone 4: Borings B-17 through B-21, B-30, SS-1 & SS-2
(reduce by 50% for upper 4 ft)

Soil Description	LPILE Soil Type	Relative Density / Consistency	Total Unit Weight (pcf*)	E_{50}	Cohesion (psf**)	Allowable Bearing Capacity (ksf***)
Silt & Clay (0-2 ft)	Clay	"Soft" to "Medium"	115	0.01	1,000	1.5

Soil Description	LPILE Soil Type	Relative Density / Consistency	Total Unit Weight (pcf*)	E_{50}	Cohesion (psf**)	Allowable Bearing Capacity (ksf***)
Silt & Clay (2-6 ft)	Clay	"Medium" to "Stiff"	115	0.01	1,500	1.5
Silty CLAY & Silty Gravel (6 ft+)	Clay	"Medium Dense/Very Stiff" to "Dense/Hard"	125	0.005	4,000	4

* *pcf – pounds per cubic foot*
** *psf – pounds per square foot*
*** *ksf – kips per square foot*

Table 6e. Summary of Unfactored Soil Parameters for Design Zone 5: Borings B-22 through B-29 (reduce by 50% for upper 4 ft)

Soil Description	LPILE Soil Type	Relative Density / Consistency	Total Unit Weight (pcf*)	Friction Angle (degrees)	E_{50}	Cohesion (psf**)	Soil Modulus, k (pci***)	Allowable Bearing Capacity (ksf****)
SILT (0-6 ft)	Clay	"Medium" to "Stiff"	115	-	0.01	1,500	-	2
SILT & SILTY SAND (6 ft+)	Sand	"Medium Dense/Very Stiff" to "Dense/Hard"	125	32	-	-	225	3

* *pcf – pounds per cubic foot*
** *psf – pounds per square foot*
*** *pci – pounds per cubic inch*
**** *ksf – kips per square foot*

We recommend that the installation of each pile size or system utilized should be monitored and documented by qualified geotechnical personnel under the direct supervision of a professional engineer registered in the State of New York. Prior to or during construction, we recommend that tension and lateral load tests be conducted on a minimum of two piles for each pile size or system planned to be utilized for this project to verify the adequacy of the design. Testing should be performed in general accordance with ASTM 3689 and ASTM 3966 or in accordance with standard practice in the industry. The test locations should coincide with the test boring locations based on the variability of the subsurface conditions. Each planned pile type should be installed with the same means and methods used to install production piles. In the event that the means and methods of pile installation are revised following initial pile testing, additional pile tests should be performed to verify that sufficient resistance can be achieved with the revised means and methods. The results should be reviewed and approved by a qualified geotechnical engineer.

4.2.3 Shallow Foundations

Shallow foundation systems such as rigid mats can be considered for support of electrical equipment. Mats supporting electrical equipment can be designed for an allowable bearing capacity of 2,000 psf when constructed in accordance with the general recommendations presented in the *Earthwork* section of this report. A vertical subgrade modulus of 100 pci may be used in foundation mat design. Foundation subgrades for supporting electrical equipment or other ancillary structures subjected to freezing temperatures during construction and/or the life of the structure should be established at least 4 ft below adjacent grades or otherwise protected against frost action. Alternatively, to resist frost heave, light loaded mat slabs constructed at grade should be provided a coarse aggregate similar to AASHTO #57 aggregate below the slab that extends to frost depth. To guard against a punching type shear failure, minimum widths of continuous footings should be 24 in.

Shallow excavations for foundation slabs and construction of utilities are not expected to encounter static groundwater. However, perched groundwater should be anticipated in excavations in low lying areas or during wet periods. If perched groundwater or surface runoff are encountered, sumps and pumps should be sufficient to control groundwater and provide stable working conditions.

4.3 Earthwork

Based on our understanding of the proposed construction, significant grading and earthwork operations are not anticipated unless material removal and replacement would be considered for support of equipment foundations. The following recommendations are provided based on the site soils encountered.

Any existing subsurface utilities which conflict with the proposed development should be removed or relocated, where applicable. In areas of backfill placement and/or construction of shallow foundations, all topsoil and organic or otherwise deleterious material should be removed before foundation construction or new fill placement. Any obstructions that would interfere with new foundation construction must be removed in their entirety from a foundation location. After stripping residual topsoil and excavation to the proposed bearing elevations for shallow mat foundations, the exposed subgrade areas should be vigorously densified with as large a compactor as is practical. Loose or unstable areas identified during the course of excavation should be densified in-place or excavated and replaced with compacted load bearing fill.

The natural soils surficial soils contain predominantly fine-grained (clay and silt) content and will be sensitive to moisture and disturbance, especially during wet periods. Therefore, they will lose considerable strength when wet or disturbed by construction equipment and

could be difficult to work with during cold or wet weather. Laboratory testing of representative samples indicates that the in-situ surficial soils are generally above their optimum moisture contents. Therefore, drying of these soils should be anticipated before reuse in compacted backfills, particularly during wet seasons. Once a subgrade has been prepared, construction traffic should be controlled in such a fashion as to minimize subgrade disturbance.

Imported load-bearing fill, if required, should consist of well-graded granular material similar to SW-GW as identified by the Unified Soil Classification System (USCS) which is not excessively moist and is free from ice and snow, roots, surface coatings, sod, loam, clay, rubbish, other deleterious or organic matter, and any particles larger than 4 inches in diameter. Alternatively, an AASHTO No. 57 coarse aggregate layer (minimum 24 inches thick) could be utilized below mat foundations supporting electrical equipment to reduce frost impacts.

All backfills fills should be placed in layers not exceeding 8 in. loose thickness. This criterion may be modified in the field depending on the conditions present at the time of construction and on the compaction equipment used. Load-bearing fills for the support of foundations should be compacted to not less than 98% of maximum dry density (ASTM D 698). All fills and backfills if utilized for areas of the solar array posts or piles, should be compacted to not less the 95% of maximum dry density. Fills in paved areas or access roads, if planned, should be compacted to not less than 95% of maximum dry density. Fills in landscaped areas should be compacted to at least 90% of maximum dry density.

The sidewalls of any confined excavations deeper than 4 ft must be sloped, benched or adequately shored per OSHA 29 CFR 1926 regulations. Trench boxes and/or sheeting could be used in conjunction with open cut slopes to permit access to confined excavations. The onsite near surface soils are classified as Type B soils according to OSHA 29 CFR 1926. Open excavations in the natural soils should not be steeper than 1H:1V if dry and 1.5H:1V if submerged.

If site grading will include cuts, especially near or beyond the depths listed in Table 1, then heavy duty excavators or dozers with ripper attachments will be required to remove the decomposed rock materials.

4.4 Trench Backfill

Bedding and pipe embedment materials to be used around underground utility or electrical conduit pipes should be well graded sand or gravel conforming to the pipe manufacturer's recommendations and should be placed and compacted in accordance with project specifications, local requirements or governing jurisdiction. General fill to be used above pipe embedment materials should be placed and compacted in accordance with the recommendations contained in this section.

Utility trenches located adjacent to footings or foundations should not extend below an imaginary 1:1 (horizontal:vertical) plane projected downward from the foundation bearing surface to the bottom edge of the trench. Where utility trenches will cross beneath footing bearing planes, the footing concrete should be deepened to encase the pipe, or the utility trench should be backfilled with sand/cement slurry or lean concrete within the foundation-bearing plane.

Depended on site grading and depth of trenches, it is noted that cobbles and or refusal to excavation equipment may be encountered during excavation of trenches. Heavy duty excavators and/or hydraulic ram attachments may need to be considered if such conditions are encountered.

4.5 Surface Drainage

Positive surface water drainage gradients at least 2 percent should be provided to direct surface water away from foundations and mat slabs towards suitable discharge facilities. Ponding of surface water should not be allowed on or adjacent to structures, slabs-on-grade, or pavements. Any rain runoff should be directed away from foundation and slabs-on-grade such as equipment pads, as applicable.

In addition, a sufficiently thick velocity dissipater, such as layer of coarse drainage aggregate of at least 3 to 4 inches in size, should be placed along water flow paths to dissipate concentrated flow of runoff water in order to minimize surface erosion.

4.6 Plans, Specifications, and Construction Review

We recommend that TRC perform a plan review of the geotechnical aspects of the project design for general conformance with our recommendations. In addition, subsurface materials encountered in the relatively small diameter, widely spaced borings may vary significantly from other subsurface materials on the site. Therefore, we also recommend that a representative of our firm observe and confirm the geotechnical specifications of the project construction. This will allow us to form an opinion about the general conformance of the project plans and construction with our recommendations. In addition, our observations during construction will enable us to note subsurface conditions that may vary from the conditions encountered during our investigation and, if needed, provide supplemental recommendations. For the above reasons, our geotechnical recommendations are contingent upon geotechnical observation and testing services by qualified geotechnical professionals during construction to confirm that site conditions do not vary considerably from the conditions previously observed. These services are not included as part of TRC's current scope of work.

4.7 Construction Observation

A qualified geotechnical professional should observe the geotechnical aspects of the earthwork for general conformance with our recommendations including site preparation, selection of fill materials, pile installation, and the placement and compaction of fill. To facilitate your construction schedule and if you wish TRC to perform these services, we request sufficient notification (72 hours) for site visits. The project plans and specifications should incorporate all recommendations contained in the text of this report. These services are not included as part of TRC's current scope of work.

5.0 LIMITATIONS

This report has been prepared ConnectGen LLC., specifically for design of the proposed solar array and associated development to be constructed at the project site located in Glen, NY, as identified herein.
Transfer of this report or included information is at the sole discretion of ConnectGen LLC. TRC's contractual relationship remains with ConnectGen LLC and limitations stated herein remain applicable regardless of end user. The opinions, conclusions, and recommendations presented in this report have been formulated in accordance with accepted geotechnical engineering practices that exist in the area at the time this report was written. No other warranty, expressed or implied, is made or should be inferred.

The opinions, conclusions and recommendations contained in this report are based upon the information obtained from our investigation, which includes data from a limited number of widely separated discrete locations, visual observations from our site reconnaissance, and review of other geotechnical data provided to us, along with local experience and engineering judgment. An attempt has been made to provide for normal contingencies; however, the possibility remains that differing or unexpected conditions may be encountered during construction. If this should occur, or if additional or contradictory data are revealed in the future, TRC should be notified so that modifications to this report can be made, if necessary. TRC is not responsible for any conclusions or opinions drawn from the data included herein, other than those specifically stated, nor are the recommendations presented in this report intended for direct use as construction specifications.

TRC should be retained to review the geotechnical aspects of the final plans and specifications for conformance with our recommendations. The recommendations provided in this report are based on the assumption that TRC will be retained to provide observation and testing services during construction to confirm that conditions are similar to that assumed for design and to form an opinion as to whether the work has been performed in accordance with the project plans and specifications. If we are not retained for these services, TRC cannot assume any responsibility for any potential claims that may arise during or after construction as a result of misuse or misinterpretation of TRC's report by

others. Furthermore, TRC will cease to be the Geotechnical Engineer-of-Record at the time another consultant is retained for follow up service to this report, if applicable.

The opinions presented in this report are valid as of the present date for the property evaluated. Changes in the condition of the property will likely occur with the passage of time due to natural processes and/or the works of man. In addition, changes in applicable standards of practice can occur as a result of legislation and/or the broadening of knowledge. Furthermore, geotechnical issues may arise that were not apparent at the time of our investigation. Accordingly, the opinions presented in this report may be invalidated, wholly or partially, by changes outside of our control. Therefore, this report is subject to review and should not be relied upon after a period of three years. Similarly, this report should not be used, nor are its recommendation applicable, for any other properties or alternate developments.

We trust this report contains the information you require and thank you for the opportunity to work on this project. Please consider our firm for future geotechnical services as needed.

Sincerely,

TRC Engineers, Inc.

James P. Benjamin, PE*
Geotechnical Project Manager
*NJ, PA

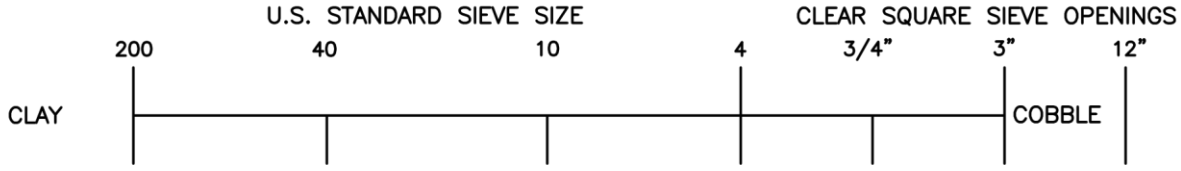
Petro W. Kazaniwsky, PE
Chief Geotechnical Engineer
NY License No.: 081310

Nhi K. Lam, EIT
Geotechnical Engineer

SILTS AND CLAY	SAND			GRAVEL		COBBLES	BOULDERS
	FINE	MEDIUM	COARSE	FINE	COARSE		

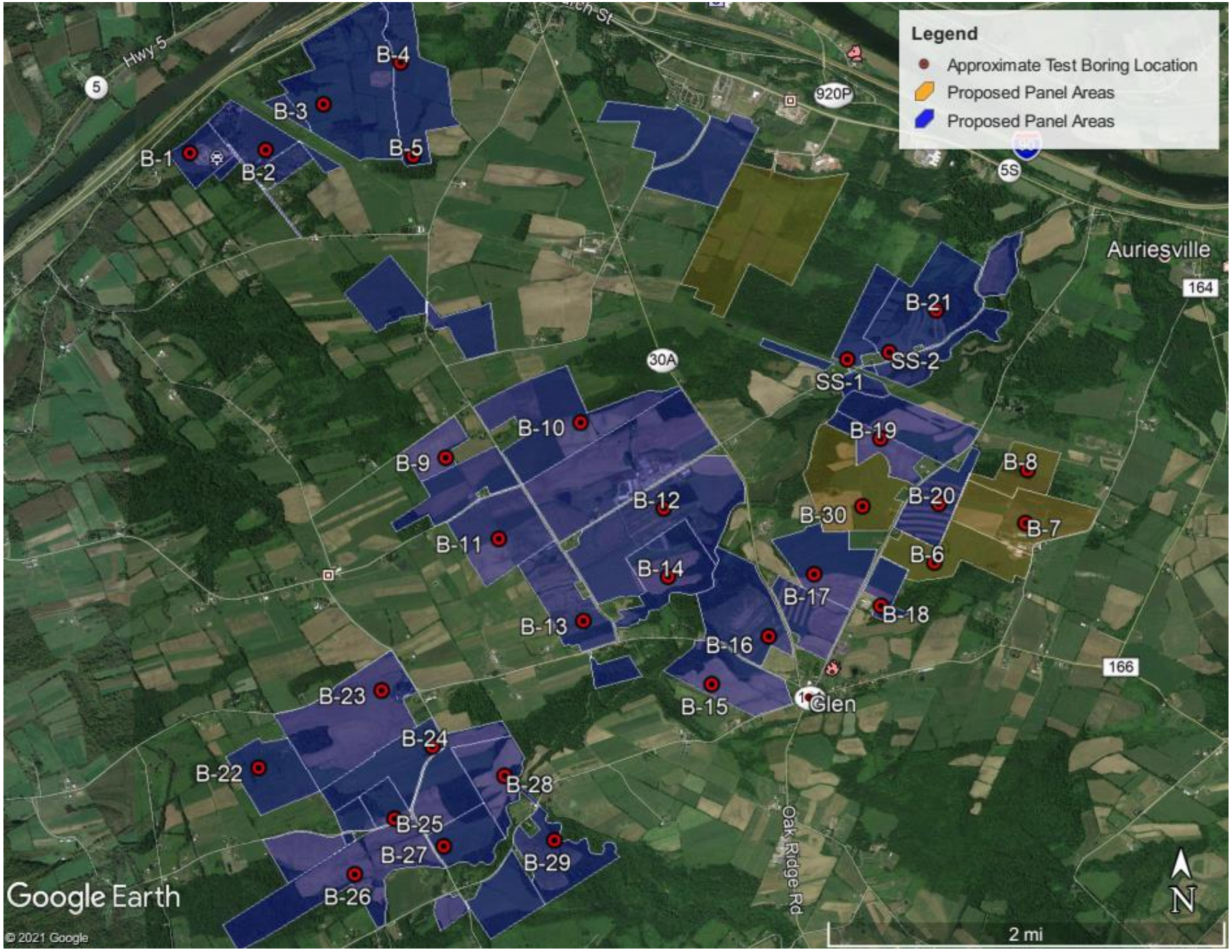
PRIMARY DIVISIONS			SOIL TYPE		SECONDARY DIVISIONS
COARSE GRAINED SOILS — MORE THAN HALF OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE	GRAVELS — MORE THAN HALF OF COARSE FRACTION IS LARGER THAN NO. 4 SIEVE	CLEAN GRAVELS (Less than 5% Fines)	GW		Well graded gravels, gravel–sand mixtures, little or no fines
			GP		Poorly graded gravels or gravel–sand mixtures, little or no fines
		GRAVEL WITH FINES	GM		Silty gravels, gravel–sand–silt mixtures, plastic fines
			GC		Clayey gravels, gravel–sand–clay mixtures, plastic fines
	SANDS — MORE THAN HALF OF COARSE FRACTION IS SMALLER THAN NO. 4 SIEVE	CLEAN SANDS (Less than 5% Fines)	SW		Well graded sands, gravelly sands, little or no fines
			SP		Poorly graded sands or gravelly sands, little or no fines
		SANDS WITH FINES	SM		Silty sands, sand–silt–mixtures, non–plastic fines
			SC		Clayey sands, sand–clay mixtures, plastic fines
FINE GRAINED SOILS — MORE THAN HALF OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE	SILTS AND CLAYS — LIQUID LIMIT IS LESS THAN 50 %		ML		Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity
			CL		Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
			OL		Organic silts and organic silty clays of low plasticity
	SILTS AND CLAYS — LIQUID LIMIT IS GREATER THAN 50 %		MH		Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
			CH		Inorganic clays of high plasticity, fat clays
			OH		Organic clays of medium to high plasticity, organic silts
HIGHLY ORGANIC SOILS			PT		Peat and other highly organic soils

DEFINITION OF TERMS



FIELD DATA

FIGURES



Project No.	411360.GEO1
Date:	June 9, 2021
For:	ConnectGen LLC Houston, TX

TRC

16000 Commerce Parkway, Mt. Laurel, New Jersey 08054
PH. (856) 273-1224 FAX. (856) 273-9244

APPROXIMATE TEST BORING LOCATIONS

Mill Point Solar

Town of Glen, Montgomery County, New York

FIGURE

1

TEST BORING LOGS

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-01**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	6.6 '
d	FROM	6.6 '	TO	6.9 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/27/2021
DATE COMPLETED	04/27/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
				0.2 ***TOPSOIL***		
	S-1	2 4 3 3				
	S-2	3 4 7 7		***BROWN SILTY CLAY, TR TO SM F/M SAND, SM ORANGE STAINING***		
5	S-3	5 9 16 24				
	S-4	19 100/0.1'		6.9		
				END OF BORING AT 6.9'		AUGER REFUSAL AT 6.9 FT
10						
15						
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-02**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/27/2021
DATE COMPLETED	04/27/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	1 4 4 5		0.2 ***TOPSOIL***		
	S-2	3 4 6 8				
5	S-3	12 12 16 12		***DARK BROWN SILTY CLAY, TR GRAVEL, SM ORANGE STAINING ON GRAVEL, MOIST***		
	S-4	19 30 23 30				
10	S-5	10 16 22 26		10.0		
				DARK BROWN TO BLACK SILTY CLAY, SM GRAVEL		
15	S-6	7 7 11		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-03**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/27/2021
DATE COMPLETED	04/27/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
				0.3 ***TOPSOIL***		
	S-1	2 3 5 5				
	S-2	4 5 6 6		***BROWN CLAYEY SILT, TR TO SM F/M SAND, MOIST***		
5	S-3	3 3 6 5		6.3		
	S-4	3 5 18 76		***BROWN CLAY, TR TO SM GRAVEL***		PROBABLE COBBLES 7.8 FT TO 8.5 FT
	S-5	100/0.4'		8.5		
10				***BROWN TO DARK GRAY SILT, TR TO SM GRAVEL, MOIST***		
15	S-6	8 10 13		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-04**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/27/2021
DATE COMPLETED	04/27/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
				0.2 ***TOPSOIL***		
	S-1	1 2 3 5				
	S-2	4 4 4 4		***DARK BROWN SAND AND CLAYEY SILT, TR TO SM F/C GRAVEL***		
5	S-3	2 8 4 15				
	S-4	17 14 20 25		7.0 8.0 ***COBBLES***		
10	S-5	10 10 13 15		***DARK GRAY SILT, SM F/M SAND, TR TO SM GRAVEL***		
15	S-6	5 7 7		15.0 ***END OF BORING AT 15'***		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-05**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/27/2021
DATE COMPLETED	04/27/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	1 1 3 6		0.2 ***TOPSOIL***		
	S-2	8 12 12 20		***DARK BROWN TO BROWN CLAYEY SILT, TR TO SM F/M/C SAND, TR GRAVEL***		SOME CLAY FROM 4 FT TO 5 FT
5	S-3	7 11 14 27				
	S-4	20 21 25 25		7.0		PROBABLE COBBLES FROM 7 FT TO 10 FT
10	S-5	9 16 87 37		***BROWN AND DARK GRAY SILTY CLAY, SM F/M/C SAND, TR TO SM GRAVEL***		
15	S-6	34 10 14		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-06**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	3.5 '
d	FROM	3.5 '	TO	7.7 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	N. LAM
DATE STARTED	05/05/2021
DATE COMPLETED	05/05/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	1 3 3 4		***DARK BROWN SILTY CLAY, TR TO SM F/M SAND, MOIST*** 2.0		
	S-2	3 7 12 26		***GRAVEL-SIZED ROCK FRAGMENTS, SM SILT,SM F/M/C SAND***		PROBABLE BOULDER AT 3.5 FT
5	S-3	26 21 18 25				
	S-4	20 24 32 100/0.2'		7.7		
				END OF BORING AT 7.7'		
10						
15						
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-07**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	N. LAM
DATE STARTED	05/05/2021
DATE COMPLETED	05/05/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	WH 1 1 4		***BROWN SILTY CLAY, TR F/ SAND, WITH ROOTS (ORGANICS)*** 2.0		
	S-2	4 6 7 9		***BROWN SILTY CLAY, TR TO SM F/ SAND, GLACIAL TILL, MOIST***		
5	S-3	7 10 14 22		6.0		
	S-4	22 26 34 35				
10	S-5	20 26 40 50		***DARK GRAY F/M/C SAND AND SILT, TR TO SM GRAVEL, MOIST***		
15	S-6	12 22 25 33		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-08**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME
12.5'	14:00	5/5	0 HR

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	N. LAM
DATE STARTED	05/05/2021
DATE COMPLETED	05/05/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	1 2 4 4				
	S-2	2 2 2 4		***DARK BROWN CLAYEY SILT, TR TO SM F/ SAND, TR F/ GRAVEL***		
5	S-3	4 5 9 6		6.0		
	S-4	6 6 5 5		***BROWN F/M/C SAND, SM F/ GRAVEL, SM SILT, WET*** 8.0		
10	S-5	3 4 4 4		***BROWN F/M/C SAND AND CLAYEY SILT, TR TO SM GRAVEL, WET*** 10.0		
				DARK GRAY SILTY CLAY, SM F/ SAND, WET		
15	S-6	WH 2 3 3		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-09**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME
10.3'	NR	4/28	0 HR

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	13.9 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/28/2021
DATE COMPLETED	04/28/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
				0.2 ***TOPSOIL***		
	S-1	3 4 4 6				
	S-2	4 4 4 7				
5	S-3	5 8 12 16		***DARK BROWN F/M/C SAND AND SILT, TR TO SM GRAVEL, SOME ORANGE STAINING***		
	S-4	11 13 13 18				
10 ▼	S-5	13 13 59 96				
				12.0		
				GRAVEL-SIZED ROCK FRAGMENTS		
	S-6	100/0.4'		13.9		
15				***END OF BORING AT 13.9'***		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-10**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/28/2021
DATE COMPLETED	04/28/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	1 2 4 7		0.2 ***TOPSOIL***		
	S-2	6 7 9 10		***BROWN SILT, SM GRAVEL, SOME ORANGE STAINING***		
5	S-3	9 12 15 15				
	S-4	12 16 19 44		7.5		PROBABLE COBBLES FROM 7.5 FT
10	S-5	37 67 61 32		***DARK GRAY TO BLACK SILT, SM GRAVEL***		
15	S-6	44 45 100		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-11**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	6.3 '
d	FROM	6.3 '	TO	7.6 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/28/2021
DATE COMPLETED	04/28/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
				0.2 ***TOPSOIL***		
	S-1	2 3 6 8				
	S-2	5 7 11 14		***BROWN CLAYEY SILT, TR TO SM GRAVEL, TR TO SM F/M/C SAND***		
5	S-3	6 14 26 35				
				6.5		
	S-4	100/0.3'		7.6 ***DECOMPOSED SHALE***		AUGER REFUSAL AT 7.6 FT
				END OF BORING AT 7.6'		
10						
15						
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-12**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/28/2021
DATE COMPLETED	04/28/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
				0.2 ***TOPSOIL***		
	S-1	2 5 7 6				
	S-2	5 7 11 13				
5	S-3	16 23 19 20				POSSBILE COBBLES FROM 5 FT
	S-4	16 20 22 24		***BROWN F/M/C SANDY SILT, TR TO SM GRAVEL***		
10	S-5	13 28 36 25				
15	S-6	7 15 22		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-13**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/28/2021
DATE COMPLETED	04/28/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	WH/1.5' 24		0.2 ***TOPSOIL***		
	S-2	5 7 9 12				
5	S-3	7 11 13 16		***BROWN, DARK BROWN AND DARK GRAY CLAYEY SILT, TR TO SM F/M SAND, TR GRAVEL, SOME ORANGE STAINING***		
	S-4	15 16 17 22				
10	S-5	6 8 10 15				ENCOUNTERED POSSIBLE COBBLES FROM 8.7 FEET TO 10 FEET
	S-6	17 10 12		15.0		
15				***END OF BORING AT 15'***		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-14**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/28/2021
DATE COMPLETED	04/28/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
				0.2 ***TOPSOIL***		POSSBILE COBBLES THROUGHOUT
	S-1	2 3 5 5				
	S-2	4 4 5 4				
5	S-3	3 6 6 14				
	S-4	12 18 14 16		***BROWN, DARK BROWN, DARK GRAY SILTY CLAY, TR TO SM F/M SAND, TR TO SM GRAVEL***		
10	S-5	20 11 19 27				
15	S-6	48 25 24		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-15**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/30/2021
DATE COMPLETED	04/30/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
				0.2 ***TOPSOIL***		
	S-1	1 4 5 7				
	S-2	6 8 9 10		***BROWN, DARK BROWN AND BLACK SILT, TR TO SM F/M SAND, TR GRAVEL***		
5	S-3	4 4 4 4				
	S-4	6 9 13 19		8.0		
	S-5	16 25 100/0.3'		***BROWN AND BLACK SILTY F/M/C SAND, SM GRAVEL***		
10						
15	S-6	45 52 61		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-16**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	N. LAM
DATE STARTED	05/03/2021
DATE COMPLETED	05/03/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	WH 2 3 4		***DARK GRAY CLAYEY SILT, TR GRAVEL, MOIST***		
	S-2	3 9 9 11		4.0		
5	S-3	10 10 14 14		***DARK GRAY CLAYEY SILT, TR TO SM GRAVEL, TR TO SM F/M/C SAND, MOIST***		
	S-4	12 18 30 35		8.0		
10	S-5	15 17 14 17		***DARK GRAY F/M/C SAND AND SILT, SM F/C GRAVEL, MOIST***		
				13.0		
	S-6	7 9 14		***DARK GRAY CLAYEY SILT, TR TO SM F/C GRAVEL***		
15				15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-17**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED 6.0 '			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	9.0 '
d	FROM	9.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	N. LAM
DATE STARTED	05/03/2021
DATE COMPLETED	05/03/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	WH 3 3 5		***BROWN SILT, TR TO SM F/M/C SAND, TR GRAVEL, WITH ORGANICS (ROOTS), MOIST***		
	S-2	4 5 7 7		4.0		
5	S-3	7 7 7 12		***DARK GRAY CLAY, SM F/M/CSAND, TR TO SM GRAVEL-SIZED ROCK FRAGMENTS, MOIST TO WET***		
	S-4	3 8 21 10				
	S-5	36 27 50/0		9.0		
10				***SHALE COBBLES***		AUGER REFUSAL AT 10.5 FT; BORING OFFSET AND COMPLETED TO 15 FT.
				13.5		
15	S-7	21 16 16		***DARK GRAY SILTY CLAY, TR F/M/C SAND, TR GRAVEL, WET*** 15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-18**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	6.0 '
d	FROM	6.0 '	TO	8.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	N. LAM
DATE STARTED	05/03/2021
DATE COMPLETED	05/03/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
5	S-1	1 1 4 6		***BROWN CLAY, TR TO SM F/M SAND, TR F/C GRAVEL, MOIST***		
	S-2	4 7 9 10				
	S-3	4 5 6 80		6.0		
				GRAVEL-SIZED SHALE FRAGMENTS 8.0		S-4: 50/0' AT 6 FT
10				***END OF BORING AT 8'***		REFUSAL AT 8 FT
15						
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-19**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	7.2 '
d	FROM	7.2 '	TO	7.5 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	N. LAM
DATE STARTED	05/04/2021
DATE COMPLETED	05/04/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	WH 1 2 4		***BROWN SILTY CLAY, TR F/M/C SAND WITH ROOTS (ORGANICS), MOIST*** 2.0		
	S-2	5 7 9 16		***BROWN SILT, TR F/M/C SAND, MOIST***		
5	S-3	6 9 12 18		6.0		
	S-4	48 80 100/0.2'		***DECOMPOSED SHALE*** 7.5		AUGER REFUSAL AT 7.5 FT
				END OF BORING AT 7.5'		
10						
15						
20						
25						
30						
35						

Boring moved 20 feet north

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-20**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	N. LAM
DATE STARTED	05/05/2021
DATE COMPLETED	05/05/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	1 2 2 3				
	S-2	3 4 4 4				
5	S-3	4 29 6 6				POSSIBLE COBBLE AT 5 FT
	S-4	6 5 5 8		***BROWN TO DARK GRAY SILTY CLAY, TR TO SM F/M/C SAND, TR GRAVEL***		
10	S-5	49 16 17 17				
15	S-6	12 14 12 9		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-21**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	N. LAM
DATE STARTED	05/04/2021
DATE COMPLETED	05/04/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	WH 2 3 3		***BROWN SILT, SM F/ SAND, SM ROOTS/ORGANICS, MOIST*** 2.0		
	S-2	3 4 6 8		***GRAYISH BROWN AND BROWN F/M/C SAND AND SILT, TR TO SM GRAVEL, MOIST***		
5	S-3	2 3 3 3		6.0		
	S-4	2 3 4 6				
10	S-5	6 7 9 13		***GRAY SILTY CLAY, TR F/M/C SAND, TR GRAVEL, MOIST***		
15	S-6	4 3 4 6		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-22**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/29/2021
DATE COMPLETED	04/29/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	WH/1.0' 2 8		0.3 ***TOPSOIL***		
	S-2	4 6 6 7		***BROWN, DARK BROWN AND BLACK SILT, SM F/M/C SAND, TR TO SM F/C GRAVEL, MOIST***		
5	S-3	4 5 10 16				
	S-4	15 16 17 17		8.8		
10	S-5	7 13 16 18		***DARK GRAY TO BLACK SILTY F/ SAND, MOIST***		
15	S-6	10 11 14		15.0 ***END OF BORING AT 15'***		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-23**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/29/2021
DATE COMPLETED	04/29/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
				0.2 ***TOPSOIL***		
	S-1	1 2 6 8		***BROWN AND DARK BROWN CLAYEY SILT, TR TO SM F/M SAND, TR GRAVEL***		
	S-2	4 4 3 4		4.0		
5	S-3	2 5 20 46		***DARK BROWN SILT, TR TO SM F/M/C SAND, TR TO SM GRAVEL-SIZED ROCK FRAGMENTS*** 6.0		
	S-4	100/0.1'		***POSSIBLE COBBLES*** 8.0		
10	S-5	24 21 26 32				
				BLACK SILT, SM F/M SAND, SM F/C GRAVEL-SIZED ROCK FRAGMENTS		
15	S-6	30 4 100/0.3'		14.8		
				END OF BORING AT 14.8'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING **B-24**
G.S. ELEV.
FILE 411360.GEO1
SHEET 1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED 10.3 '			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER B. FLETCHER
HELPER R. CASATELLI
INSPECTOR A. FISHMAN
DATE STARTED 04/29/2021
DATE COMPLETED 04/29/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	1 1 41 4		0.2 ***TOPSOIL***		
	S-2	6 11 12 20				
5	S-3	8 15 17 23		***BROWN SILT, SM F/M/C SAND, TR TO SM F/ GRAVEL***		
	S-4	18 26 28 24				
10 ▽	S-5	8 14 15 19				SAND SEAM FROM 9 FT TO 9.3 FT
				14.3		
15	S-6	7 9 12		15.0 ***GRAY SILTY F/M SAND***		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN. SAP
CKD. JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING **B-25**
G.S. ELEV.
FILE 411360.GEO1
SHEET 1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED 14.2 '			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER B. FLETCHER
HELPER R. CASATELLI
INSPECTOR A. FISHMAN
DATE STARTED 04/29/2021
DATE COMPLETED 04/29/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
				0.2 ***TOPSOIL***		
	S-1	1 3 4 3				
	S-2	3 3 3 4				
5	S-3	4 3 3 4				SAND SEAM FROM 5.8 FT TO 6 FT
	S-4	6 8 13 16		***BROWN, DARK GRAY AND BLACK F/M SANDY SILT, TR GGRAVEL-SIZED ROCK FRAGMENTS***		
10	S-5	8 10 12 13				
▽ 15	S-6	13 20 27		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN. SAP
CKD. JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-26**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/30/2021
DATE COMPLETED	04/30/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
				0.2 ***TOPSOIL***		
	S-1	1 5 5 6				
	S-2	5 9 7 51				
5	S-3	4 6 7 10				
	S-4	8 12 17 22		***BROWN, DARK GRAY AND BLACK SILT, SM F/M SAND, SM GRAVEL-SIZED ROCK FRAGMENTS***		
10	S-5	15 23 23 37				
15	S-6	13 18 22		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-27**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	15.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/30/2021
DATE COMPLETED	04/30/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	1 2 3 4		0.2 ***TOPSOIL***		
	S-2	4 4 4 4				
5	S-3	3 3 4 10				
	S-4	16 32 14 15		***BROWN SILT, SM F/M/C SAND, TR TO SM GRAVEL-SIZED SHALE FRAGMENTS***		
10	S-5	12 16 19 29				
						COBBLES PRESENT FROM 13.5-15 FEET
15	S-6	15 11 14		15.0		
				END OF BORING AT 15'		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING **B-28**
G.S. ELEV.
FILE 411360.GEO1
SHEET 1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED 3.0 '			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	9.4 '
d	FROM	9.4 '	TO	15.0 '

DRILLER B. FLETCHER
HELPER R. CASATELLI
INSPECTOR A. FISHMAN
DATE STARTED 04/30/2021
DATE COMPLETED 04/30/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	2 4 6 4		0.2 ***TOPSOIL***		
	S-2	8 3 4 7		***BROWN SILT, SM F/M/C SAND, TR TO SM GRAVEL-SIZED SHALE FRAGMENTS***		
5	S-3	5 4 2 8				
	S-4	12 16 19 22		8.0		
	S-5	57 84 100/0.4'		***BLACK CLAYEY SILT, TR TO SM GRAVEL-SIZED ROCK FRAGMENTS***		
10						
15	S-6	67 89 41		15.0 ***END OF BORING AT 15'***		
20						
25						
30						
35						

DRN. SAP
CKD. JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-29**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	9.4 '
d	FROM	9.4 '	TO	14.1 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	A. FISHMAN
DATE STARTED	04/30/2021
DATE COMPLETED	04/30/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	WH/1.0' 2/1.0'		0.2 ***TOPSOIL***		
	S-2	2 2 4 5		***BROWN SILT, TR TO SM GRAVEL-SIZED ROCK FRAGMENTS***		
5	S-3	5 2 3 3		6.6		
	S-4	11 32 25 50/0.4'				
	S-5	32 66 100/0.4'		***BLACK SILTY F/M/C SAND, SM GRAVEL-SIZED SHALE FRAGMENTS***		
10						
	S-6	50 77 100/0.1'		14.6		
15				***END OF BORING AT 14.6'***		
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**B-30**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME
2.5'	16:15	5/4	0 HR

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	6.5 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	N. LAM
DATE STARTED	05/04/2021
DATE COMPLETED	05/04/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	3 5 7 8		***BROWN SILT, TR TO SM F/M/C SAND, TR GRAVEL***		
	S-2	17 9 7 7		4.0		
5	S-3	3 3 36 48		***DARK BROWN SILTY GRAVEL, TR TO SM F/M/C SAND, WET*** 6.0		
	S-4	20 50/0		6.5 ***DECOMPOSED SHALE***		AUGER REFUSAL AT 6.5 FT
				END OF BORING AT 6.5'		
10						
15						
20						
25						
30						
35						

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**SS-01**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED 18.0 '			
DEPTH	HOUR	DATE	ELAPSED TIME

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	35.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	N. LAM
DATE STARTED	05/03/2021
DATE COMPLETED	05/04/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	1 2 5 5				
	S-2	6 12 14 15				
5	S-3	7 10 18 27		***BROWN CLAY, SM F//M/C SAND, TR TO SM GRAVEL, MOIST***		
	S-4	21 32 44 35				
10	S-5	13 21 23 24				
				13.0		
15	S-6	12 17 20		***DARK GRAY F/M/C SANDY SILT, TR F/ GRAVEL, MOIST TO WET***		WET FROM 18 FT TO 35 FT
▽						
20	S-7	33 21 27 36				
				23.0		
25	S-8	54 15 27 46		***DARK GRAY SILTY CLAY, TR TO SM F/M/C SAND, TR F/ GRAVEL, WET***		
30	S-9	29 27 28 25				
35	S-10			35.0		

END OF BORING AT 35'

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

TEST BORING LOG

PROJECT: MILL POINT SOLAR AND BATTERY STORAGE PROJECT
LOCATION: GLEN, NY

BORING	**SS-02**
G.S. ELEV.	
FILE	411360.GEO1
SHEET	1 OF 1

GROUNDWATER DATA			
FIRST ENCOUNTERED N/A			
DEPTH	HOUR	DATE	ELAPSED TIME
8.2'	NR	5/4	0 HR

METHOD OF ADVANCING BOREHOLE				
a	FROM	0.0 '	TO	10.0 '
d	FROM	10.0 '	TO	35.0 '

DRILLER	B. FLETCHER
HELPER	R. CASATELLI
INSPECTOR	N. LAM
DATE STARTED	05/04/2021
DATE COMPLETED	05/04/2021

DEPTH	A	B	C	DESCRIPTION	PP	REMARKS
	S-1	WH 3 4 6		***BROWN SILTY CLAY AND F/M/C SAND, TR TO SM F/C GRAVEL, MOIST***		
	S-2	5 6 10 11				
5	S-3	5 26 18 16		2.0 ***BROWN SILT, SM F/M/C SAND, TR GRAVEL, MOIST***		
	S-4	8 15 24 25		6.0		
10	S-5	76 54 48 39		***BROWN F/M/C SANDY SILT, TR GRAVEL, MOIST***		PROBABLE COBBLES FROM 8-11 FT
				13.0		
15	S-6	26 28 21 26				
				DARK GRAY CLAY, TR TO SM F/M/C SAND, TR TO SM F/C GRAVEL, MOIST TO WET		
20	S-7	28 17 20 33				
				23.0		
25	S-8	9 11 17 22		***DARK GRAY SILTY CLAY, TR TO SM F/M SAND***		
30	S-9	10 12 16 21				
35	S-10	10 12 20 23		35.0		

END OF BORING AT 35'

DRN.	SAP
CKD.	JPB

NEW PROJECTS TEST BORING LOG 411360.GEO1.GPJ SITE BLAUVELT.GDT 6/9/21

KEY TO SYMBOLS

Symbol Description

Strata symbols


Boulders / Cobbles


Clay with High Plasticity


Clay with Low Plasticity


Clayey Silt


Silty Clay


Highly Weathered or Decomposed Rock


Silty Gravel


Poorly-graded Gravel with Silt


Silt with Low Plasticity


USCS Gravelly Silt


USCS Sandy Silt


Silty Sand


Poorly graded silty fine sand


Poorly-graded Sand with Clay

Symbol Description

Misc. Symbols


Water table first encountered


Water table first reading after drilling


Water table second reading after drilling


Water table third reading after drilling

NR Not Recorded

MH Moh's Hardness

Sample Type


Split Barrel

Lab Symbols

FINES = Fines %

LL = Liquid Limit %

PI = Plasticity Index %

U_c = Unconfined Compressive Strength

W/V = Unit Weight

Notes:

COLUMN A) Soil sample number.

COLUMN B) FOR SOIL SAMPLE (ASTM D 1586): indicates number of blows obtained for each 6 ins. penetration of the standard split-barrel sampler. FOR ROCK CORING (ASTM D2113): indicates percent recovery (REC) per run and rock quality designation (RQD). RQD is the % of rock pieces that are 4 ins. or greater in length in a core run.

COLUMN C) Strata symbol as assigned by the geotechnical engineer.

DESCRIPTION) Description including color, texture and classification of subsurface material as applicable (see Descriptive Terms). Estimated depths to bottom of strata as interpolated from the borings are also shown.

DESCRIPTIVE TERMS: F = fine M = medium C = coarse

RELATIVE PROPORTIONS:

-Descriptive Term-	-Symbol-	-Est. Percentages-
Trace	TR	1-10
Trace to Some	TR to SM	10-15
Some	SM	15-30
Silty, Sandy, Clayey, Gravelly	-	30-40
And	and	40-50

REMARKS) Special conditions or test data as noted during investigation. Note that W.O.P. indicates water observation pipes.

* Free water level as noted may not be indicative of daily, seasonal, tidal, flood, and/or long term fluctuations.

FIELD RESISTIVITY DATA

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___ Dry _x__ Wet ___ Ideal	Date Completed:	4/30/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- yes		Helper:	NA

Test	Electrode Spacing (ft)	Resistance ⏚ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 1	2.5	12.3	5,889	
	5.0	9.3	8,943	
	10.0	9.2	17,695	
	20.0	8.5	32,708	
	50.0	7.0	67,025	

Line 1 Direction:
- ____ N-S
- ____ NE_SW
- x E-W
- ____ NW-SE

Test Location B1

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___Dry x Wet ___Ideal	Date Completed:	4/30/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- yes		Helper:	NA

Test	Electrode Spacing (ft)	Resistance ⏚ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 2	2.5	12.6	6,032	
	5.0	9.7	9,326	
	10.0	8.8	16,775	
	20.0	8.7	33,168	
	50.0	7.2	69,132	

Line 2 Direction:
- x N-S
- ____ NE_SW
- ____ E-W
- ____ NW-SE

Test Location B1

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	__ Dry _x__Wet __ Ideal	Date Completed:	4/30/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- yes		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 1	2.5	10.3	4,931	Move from B4 due to access issue
	5.0	6.5	6,262	
	10.0	4.2	8,062	
	20.0	2.7	10,226	
	50.0	1.8	16,852	

Line 1 Direction:
- _____ N-S
- _____ NE_SW
- x E-W
- _____ NW-SE

Test Location B5

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___Dry x Wet ___Ideal	Date Completed:	4/30/2021
Ambient Temperature:	70o F	Operator:	N.Lam
Rain storms previous day- yes		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 2	2.5	17.0	8,139	
	5.0	9.0	8,646	
	10.0	5.1	9,747	
	20.0	3.1	11,720	
	50.0	1.9	18,480	

Line 2 Direction:
- x N-S
- _____ NE_SW
- _____ E-W
- _____ NW-SE

Test Location B5

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___ Dry ___Wet _x_ Ideal	Date Completed:	4/28/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- No		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 1	2.5	14.7	7,038	
	5.0	7.8	7,469	
	10.0	4.8	9,211	
	20.0	3.0	11,567	
	50.0	1.8	16,852	

Line 1 Direction:	____	N-S
	____	NE_SW
	x	E-W
	____	NW-SE

Test Location B9

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___Dry ___Wet _x_ Ideal	Date Completed:	4/28/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- No		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 2	2.5	14.6	6,990	
	5.0	8.0	7,622	
	10.0	4.7	9,020	
	20.0	3.2	12,179	
	50.0	1.7	16,086	

Line 2 Direction:	____	N-S
	____	NE_SW
	x	E-W
	____	NW-SE

Test Location B9

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___ Dry ___Wet _x_ Ideal	Date Completed:	4/28/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- No		Helper:	NA

Test	Electrode Spacing (ft)	Resistance ⌖ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 1	2.5	14.2	6,798	
	5.0	7.6	7,287	
	10.0	4.2	7,986	
	20.0	2.1	8,043	
	50.0	1.2	11,682	

Line 1 Direction:
- ____ N-S
- ____ NE_SW
- x E-W
- ____ NW-SE

Test Location B12

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___Dry ___Wet x Ideal	Date Completed:	4/28/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- No		Helper:	NA

Test	Electrode Spacing (ft)	Resistance ⌖ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 2	2.5	18.6	8,905	
	5.0	7.8	7,488	
	10.0	4.1	7,928	
	20.0	2.3	8,732	
	50.0	1.2	11,490	

Line 2 Direction:
- x N-S
- ____ NE_SW
- ____ E-W
- ____ NW-SE

Test Location B12

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___ Dry ___Wet _x_ Ideal	Date Completed:	4/28/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- No		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 1	2.5	7.7	3,677	
	5.0	3.0	2,834	
	10.0	2.0	3,811	
	20.0	1.5	5,822	
	50.0	1.1	10,054	

Line 1 Direction:
- ____ N-S
- ____ NE_SW
- x E-W
- ____ NW-SE

Test Location B13

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___Dry ___Wet _x_ Ideal	Date Completed:	4/28/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- No		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 2	2.5	6.8	3,275	
	5.0	3.1	2,968	
	10.0	1.9	3,658	
	20.0	1.5	5,554	
	50.0	1.2	11,011	

Line 2 Direction:
- x N-S
- ____ NE_SW
- ____ E-W
- ____ NW-SE

Test Location B13

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___ Dry ___Wet _x_ Ideal	Date Completed:	4/28/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- No		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 1	2.5	10.5	5,027	
	5.0	6.1	5,812	
	10.0	3.8	7,334	
	20.0	2.3	8,847	
	50.0	1.4	13,788	

Line 1 Direction: x N-S; NE_SW; E-W; NW-SE

Test Location B16

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___Dry ___Wet _x_ Ideal	Date Completed:	4/28/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- No		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 2	2.5	11.0	5,266	
	5.0	6.1	5,860	
	10.0	3.7	7,124	
	20.0	2.4	9,115	
	50.0	1.5	13,884	

Line 2 Direction: N-S; NE_SW; x E-W; NW-SE

Test Location B16

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___ Dry _x__ Wet ___ Ideal	Date Completed:	4/29/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- yes		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 1	2.5	16.6	7,947	
	5.0	8.4	8,053	
	10.0	4.5	8,694	
	20.0	2.9	10,992	
	50.0	2.3	22,406	

Line 1 Direction:		N-S	**Test Location**	B20
		NE_SW		
	x	E-W		
		NW-SE		

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	Dry x Wet Ideal	Date Completed:	4/29/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- yes		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 2	2.5	16.4	7,852	
	5.0	8.2	7,890	
	10.0	4.4	8,407	
	20.0	2.7	10,494	
	50.0	2.2	21,448	

Line 2 Direction:	x	N-S	**Test Location**	B20
		NE_SW		
		E-W		
		NW-SE		

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___ Dry _x__ Wet ___ Ideal	Date Completed:	4/30/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- yes		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 1	2.5	8.6	4,098	
	5.0	5.0	4,749	
	10.0	3.3	6,320	
	20.0	1.4	5,285	
	50.0	0.6	6,128	

Line 1 Direction: x N-S; NE_SW; E-W; NW-SE

Test Location B22

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	Dry x Wet Ideal	Date Completed:	4/30/2021
Ambient Temperature:	70o F	Operator:	N.Lam
Rain storms previous day- yes		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 2	2.5	7.8	3,753	
	5.0	5.8	5,563	
	10.0	3.0	5,783	
	20.0	1.5	5,630	
	50.0	0.7	6,320	

Line 2 Direction: N-S; NE_SW; x E-W; NW-SE

Test Location B22

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___ Dry _x__ Wet ___ Ideal	Date Completed:	4/29/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- yes		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 1	2.5	8.2	3,916	
	5.0	4.3	4,127	
	10.0	3.1	5,898	
	20.0	2.2	8,464	
	50.0	1.5	14,267	

Line 1 Direction: x N-S; NE_SW; E-W; NW-SE

Test Location B27

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	Dry x Wet Ideal	Date Completed:	4/29/2021
Ambient Temperature:	70° F	Operator:	N.Lam
Rain storms previous day- yes		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 2	2.5	8.3	3,950	
	5.0	5.2	4,960	
	10.0	3.5	6,607	
	20.0	2.7	10,226	
	50.0	1.9	17,810	

Line 2 Direction: x N-S; NE_SW; E-W; NW-SE

Test Location B27

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___ Dry _x__ Wet ___ Ideal	Date Completed:	4/29/2021
Ambient Temperature:	65° F	Operator:	N.Lam
Rain storms previous day- yes		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 1	2.5	7.1	3,390	
	5.0	4.6	4,366	
	10.0	3.1	5,994	
	20.0	2.1	8,043	
	50.0	1.1	10,533	

Line 1 Direction:

____	N-S
____	NE_SW
x	E-W
____	NW-SE

Test Location SS-2

TRC Engineers, Inc.
Field Resistivity Testing
Wenner Method

Project:	Mill Point Solar	Project No.:	411360.GEO1
Location:	Glen, NY	Client:	ConnectGen
Site Conditions:	___Dry x Wet ___Ideal	Date Completed:	4/29/2021
Ambient Temperature:	65° F	Operator:	N.Lam
Rain storms previous day- yes		Helper:	NA

Test	Electrode Spacing (ft)	Resistance Φ (Ohms)	Apparent Resistivity (Ohm-cm)	Remarks
Line 2	2.5	7.5	3,567	
	5.0	4.3	4,089	
	10.0	3.2	6,166	
	20.0	2.1	7,966	
	50.0	0.9	9,001	

Line 2 Direction:

____	N-S
____	NE_SW
x	E-W
____	NW-SE

Test Location SS-2

LABORATORY DATA

SUMMARY OF LABORATORY TEST DATA

Project Name: Mill Point Solar Project
Client Name: ConnectGen, LLC
TRC Project #: 411360.GEO1

SAMPLE IDENTIFICATION			Soil Group (USCS System)	Moisture Content (%)	Dry Unit Weight (pcf)	GRAIN SIZE DISTRIBUTION				PLASTICITY			
Boring #	Sample #	Depth (ft)				Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Liquidity Index (%)
B-1 to B-5	BULK 1	0.0-5.0	CL-ML	16.2	-	6.8	40.7	52.5		22	16	6	0.0
B-1	S-2	2.0-4.0	CL*	16.6	-	-	-	-		24	16	8	0.1
B-2	S-5	8.0-10.0	CL-ML*	9.5	125.0	-	-	-		21	14	7	-0.6
B-3	S-4	6.0-8.0	CL*	18.5	-	-	-	-		25	16	9	0.3
B-4	S-3	4.0-6.0	SM	11.6	-	18.9	36.7	44.4		-	-	-	-
B-5	S-5	8.0-10.0	CL-ML*	10.7	-	-	-	-		20	13	7	-0.3
B-6 to B-8	BULK 2	0.0-5.0	CL	25.7	-	14.4	27.3	58.3		34	23	11	0.2
B-7	S-3	4.0-6.0	CL-ML*	12.7	-	-	-	-		21	15	6	-0.4

DRAWN BY: TBT 06/14/21

CHECKED BY: JPB 06/14/21

SUMMARY OF LABORATORY TEST DATA

Project Name: Mill Point Solar Project
Client Name: ConnectGen, LLC
TRC Project #: 411360.GEO1

SAMPLE IDENTIFICATION			Soil Group (USCS System)	Moisture Content (%)	Dry Unit Weight (pcf)	GRAIN SIZE DISTRIBUTION				PLASTICITY			
Boring #	Sample #	Depth (ft)				Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Liquidity Index (%)
B-7	S-4	6.0-8.0	SM	8.5	-	12.6	39.4	48.0		-	-	-	-
B-8	S-4	6.0-8.0	SM	12.9	-	24.9	47.5	27.6		-	-	-	-
B-9 to B-14	BULK 3	0.0-5.0	CL-ML	12.0	-	9.1	37.5	53.4		20	14	6	-0.3
B-9	S-4	6.0-8.0	SM	12.3	-	15.0	38.8	46.2		-	-	-	-
B-12	S-4	6.0-8.0	ML	10.6	124.8	11.1	34.6	54.3		-	-	-	-
B-12	S-6	13.0-15.0	ML	10.6	123.6	7.6	27.8	64.6		-	-	-	-
B-14	S-3	4.0-6.0	CL-ML*	15.0	-	-	-	-		23	18	5	-0.6
B-15	S-6	13.0-15.0	SM	6.6	-	27.5	33.0	39.5		-	-	-	-

DRAWN BY: TBT 06/14/21

CHECKED BY: JPB 06/14/21

SUMMARY OF LABORATORY TEST DATA

Project Name: Mill Point Solar Project
Client Name: ConnectGen, LLC
TRC Project #: 411360.GEO1

SAMPLE IDENTIFICATION			Soil Group (USCS System)	Moisture Content (%)	Dry Unit Weight (pcf)	GRAIN SIZE DISTRIBUTION				PLASTICITY			
Boring #	Sample #	Depth (ft)				Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Liquidity Index (%)
B-16	S-5	8.0-10.0	SM	8.9	-	22.1	30.5	47.5		-	-	-	-
B-17	S-3	6.0-8.0	CL*	24.7	-	-	-	-		40	24	16	0.0
B-18	S-2	2.0-4.0	CL*	25.4	-	-	-	-		42	26	16	0.0
B-19	S-2	4.0-6.0	ML*	27.9	93.9	-	-	-		48	28	20	0.0
B-20	S-5	8.0-10.0	CL-ML*	9.8	-	-	-	-		19	13	6	-0.5
B-21	S-3	4.0-6.0	ML	22.7	-	15.5	29.9	54.6		-	-	-	-
B-21	S-5	8.0-10.0	CH*	29.2	95.1	-	-	-		52	27	25	0.1
B-22 to B-29	BULK 4	0.0-5.0	CL-ML	19.7	-	2.0	21.5	76.5		18	14	4	1.4

DRAWN BY: TBT 06/14/21

CHECKED BY: JPB 06/14/21

SUMMARY OF LABORATORY TEST DATA

Project Name: Mill Point Solar Project
Client Name: ConnectGen, LLC
TRC Project #: 411360.GEO1

SAMPLE IDENTIFICATION			Soil Group (USCS System)	Moisture Content (%)	Dry Unit Weight (pcf)	GRAIN SIZE DISTRIBUTION				PLASTICITY			
Boring #	Sample #	Depth (ft)				Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Liquidity Index (%)
B-25	S-3	4.0-6.0	ML	31.4	-	1.9	31.1	67.0		-	-	-	-
B-27	S-4	6.0-8.0	ML	17.9	-	16.8	27.2	56.0		-	-	-	-
B-29	S-5	8.0-9.4	SM	5.4	-	18.7	44.6	36.7		-	-	-	-
B-30	S-3	4.0-6.0	GM	31.9	-	44.4	10.7	44.9		-	-	-	-
SS-1 & SS-2	BULK 5	0.0-5.0	CL	40.1	-	1.2	25.1	73.7		43	25	18	0.8
SS-1	S-3	4.0-6.0	CL*	19.5	-	-	-	-		32	20	12	0.0
SS-1	S-6	13.0-15.0	ML	9.6	-	9.1	31.8	59.1		-	-	-	-
SS-1	S-8	23.0-25.0	CL-ML	14.5	-	6.9	18.9	74.2		-	-	-	-

DRAWN BY: TBT 06/14/21

CHECKED BY: JPB 06/14/21

SUMMARY OF LABORATORY TEST DATA

Project Name: Mill Point Solar Project
Client Name: ConnectGen, LLC
TRC Project #: 411360.GEO1

SAMPLE IDENTIFICATION			Soil Group (USCS System)	Moisture Content (%)	Dry Unit Weight (pcf)	GRAIN SIZE DISTRIBUTION				PLASTICITY			
Boring #	Sample #	Depth (ft)				Gravel (%)	Sand (%)	Silt (%)	Clay (%)	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Liquidity Index (%)
SS-2	S-4	6.0-8.0	ML	12.4	-	4.0	33.1	62.9		-	-	-	-
SS-2	S-6	13.0-15.0	CL*	10.3	-	-	-	-		21	13	8	-0.3

DRAWN BY: TBT 06/14/21

CHECKED BY: JPB 06/14/21

SUMMARY OF LABORATORY TEST DATA

Project Name: Mill Point Solar Project
Client Name: ConnectGen, LLC
TRC Project #: 411360.GEO1

CORROSIVITY ANALYSIS OF SOILS							
Specimen Identification			pH Analysis ASTM D4972 (IN H20)	pH Analysis ASTM D4972 (IN CaCl2)	Water Soluble Sulfates, mg/kg ASTM D516	Chlorides, mg/kg ASTM D512	Resistivity, ohm-cm ASTM G57
Source #	Sample #	Depth (ft)					
B-1 to B-5	BULK 1	0.0-5.0	7.7	7.4	58	50	2,548
B-6 to B-8	BULK 2	0.0-5.0	8.1	7.5	55	40	2,940
B-9 to B-14	BULK 3	0.0-5.0	8.4	7.5	220	40	1,260
B-22 to B-29	BULK 4	0.0-5.0	8.2	8.0	235	38	1,176
SS-1 & SS-2	BULK 5	0.0-5.0	8.3	8.0	185	75	1,568

DRAWN BY: TBT 06/14/21

CHECKED BY: JPB 06/14/21

SUMMARY OF LABORATORY TEST DATA

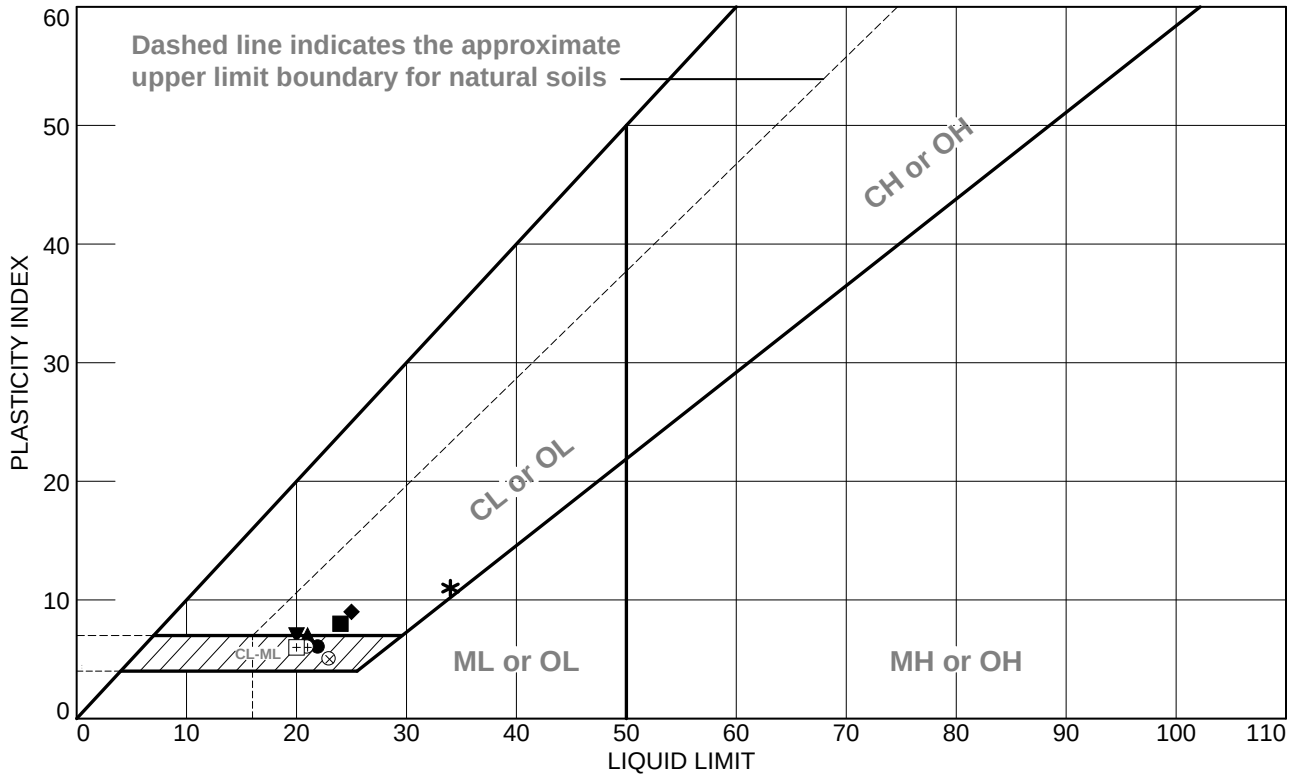
Project Name: Mill Point Solar Project
Client Name: ConnectGen, LLC
TRC Project #: 411360.GEO1

COMPACTION & THERMAL RESISTIVITY RESULTS									
Specimen Identification			**Compaction Characteristics**			**Thermal Resistivity (°C-cm/W)**		**Moisture Content (%)**	**Dry Density (pcf)**
Source #	Sample #	Depth (ft)	Type of Test	Maximum Density (PCF)	Optimum Moisture Content (%)	Wet	Dry		
B-1 to B-5	BULK 1	0.0-5.0	D698	118.6	11.8	111.5	565.0	9.0	106.3
B-6 to B-8	BULK 2	0.0-5.0	D698	109.0	14.8	61.6	120.0	14.8	98.1
B-9 to B-14	BULK 3	0.0-5.0	D698	119.0	11.9	46.8	100.0	11.9	107.1
B-22 to B-29	BULK 4	0.0-5.0	D698	114.5	12.1	69.9	174.6	12.1	103.1
SS-1 & SS-2	BULK 5	0.0-5.0	D698	96.2	18.4	68.9	182.6	18.4	86.6

DRAWN BY: TBT 06/14/21

CHECKED BY: JPB 06/14/21

LIQUID AND PLASTIC LIMITS TEST REPORT



SOIL DATA

	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	LIQUIDITY INDEX	USCS
●	B-1 TO B-5	BULK		16.2	16	22	6	0.0	CL-ML
■	B-1	S-2	2.0-4.0 FT	16.6	16	24	8	0.1	CL*
▲	B-2	S-5	8.0-10.0 FT	9.5	14	21	7	-0.6	CL-ML*
◆	B-3	S-4	6.0-8.0 FT	18.5	16	25	9	0.3	CL*
▼	B-5	S-5	8.0-10.0 FT	10.7	13	20	7	-0.3	CL-ML*
✱	B-6 TO B-8	BULK		25.7	23	34	11	0.2	CL
⊕	B-7	S-3	4.0-6.0 FT	12.7	15	21	6	-0.4	CL-ML*
⊞	B-9 TO B-14	BULK		12.0	14	20	6	-0.3	CL-ML
⊗	B-14	S-3	4.0-6.0 FT	15.0	18	23	5	-0.6	CL-ML*

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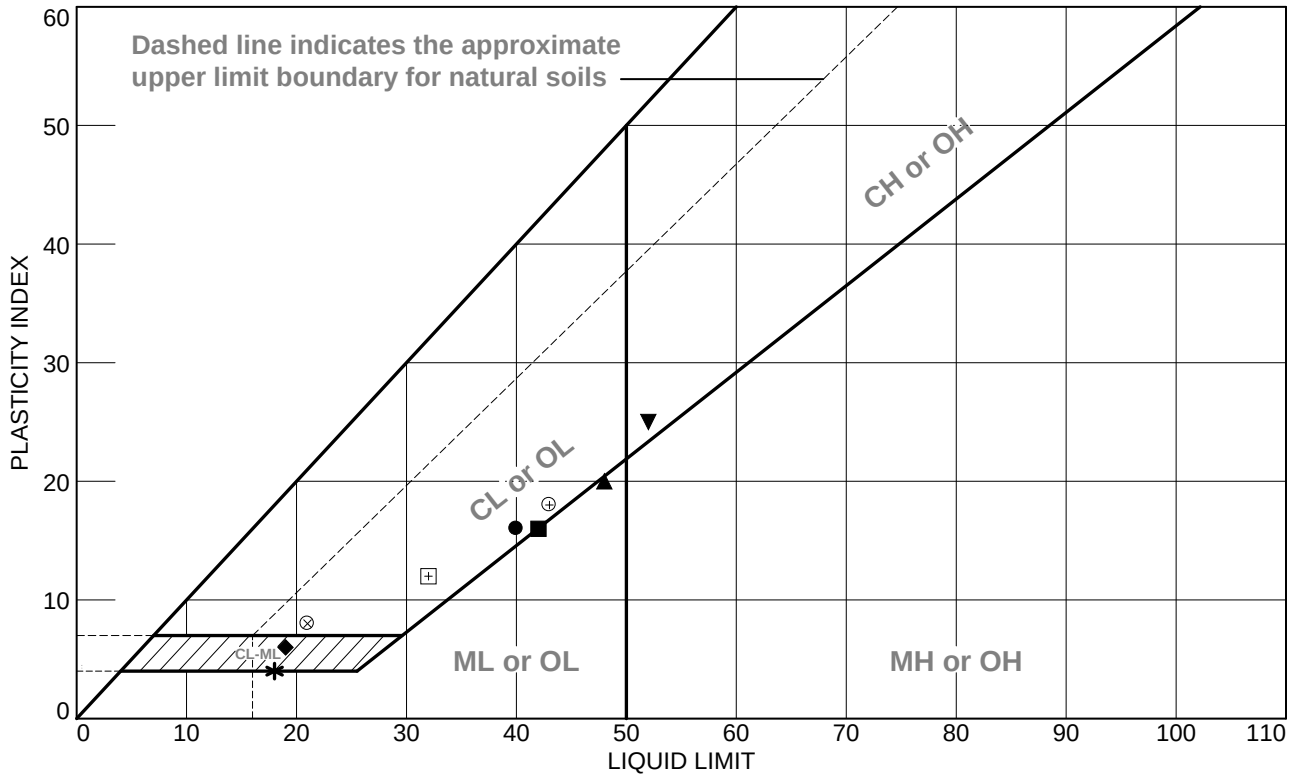
Client: CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

Project No.: 411360.GEO1 **Figure** 1

LIQUID AND PLASTIC LIMITS TEST REPORT



SOIL DATA

	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	LIQUIDITY INDEX	USCS
●	B-17	S-3	6.0-8.0 FT	24.7	24	40	16	0.0	CL*
■	B-18	S-2	2.0-4.0 FT	25.4	26	42	16	0.0	CL*
▲	B-19	S-2	2.0-4.0 FT	27.9	28	48	20	0.0	ML*
◆	B-20	S-5	8.0-10.0 FT	9.8	13	19	6	-0.5	CL-ML*
▼	B-21	S-5	8.0-10.0 FT	29.2	27	52	25	0.1	CH*
✱	B-22 TO B-29	BULK 4	0.0-5.0 FT	19.7	14	18	4	1.4	CL-ML
⊕	SS-1 & SS-2	BULK 5	0.0-5.0 FT	40.1	25	43	18	0.8	CL
⊞	SS-1	S-3	4.0-6.0 FT	19.5	20	32	12	0.0	CL*
⊗	SS-2	S-6	13.0-15.0 FT	10.3	13	21	8	-0.3	CL*

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Mt. Laurel, NJ

Client: CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

Project No.: 411360.GEO1 **Figure** 2

Particle Size Distribution Report

	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	6.8	4.6	12.1	24.0	52.5	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○	22	16	1.1328	0.1226						

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SANDY SILTY CLAY	05/24/21	CL-ML	16.2

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-1 TO B-5 **Sample Number:** BULK

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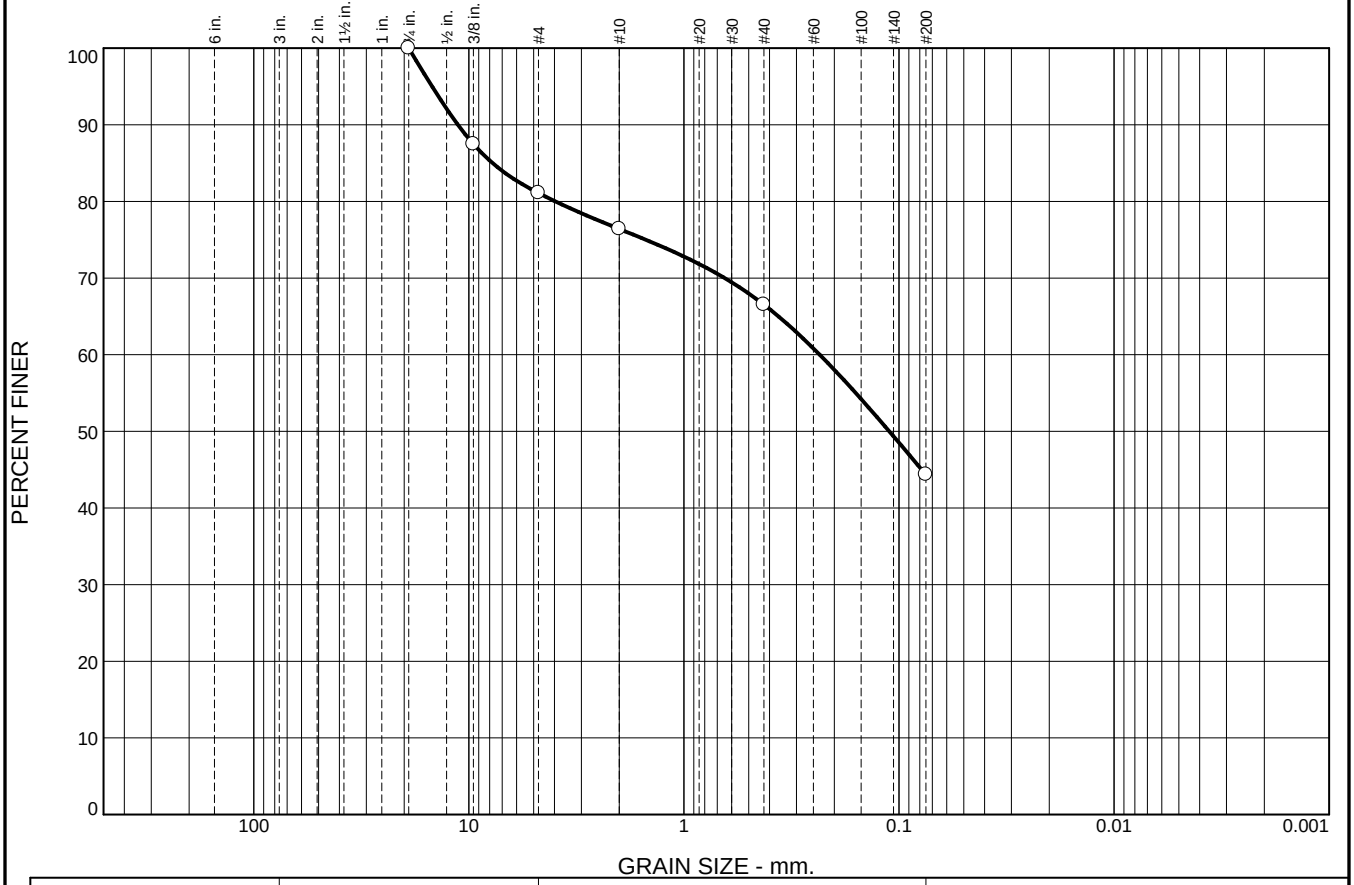
Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

Figure 3

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	18.9	4.7	9.9	22.1	44.4	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			7.7606	0.2338	0.1110					

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SILTY SAND WITH GRAVEL	05/24/21	SM	11.6

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-4 **Depth:** 4.0-6.0 FT **Sample Number:** S-3

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

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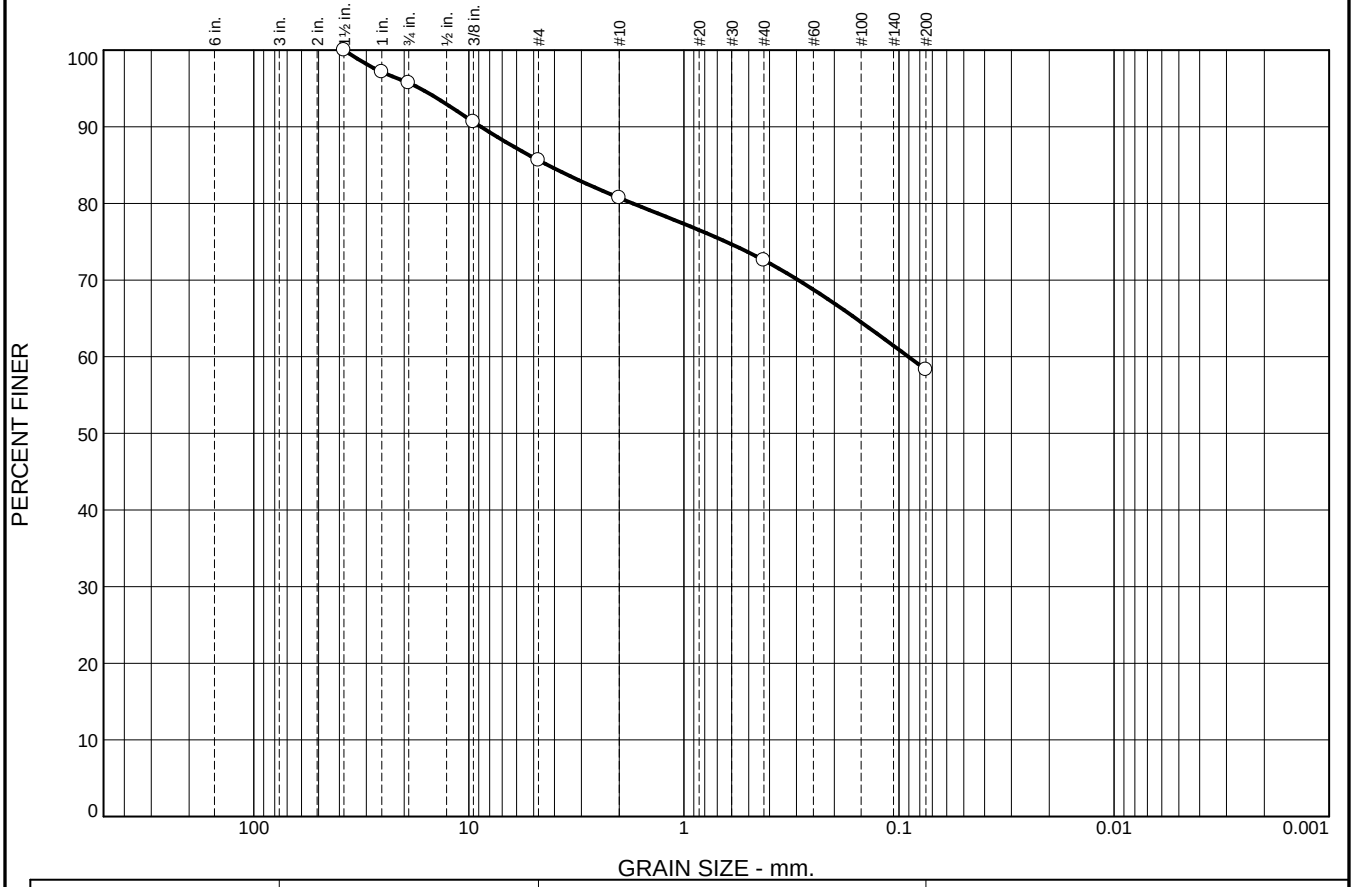
Figure 4

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	4.3	10.1	4.9	8.1	14.3	58.3	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○	34	23	4.2931	0.0904						

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SANDY LEAN CLAY	05/24/21	CL	25.7

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-6 TO B-8 **Sample Number:** BULK

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

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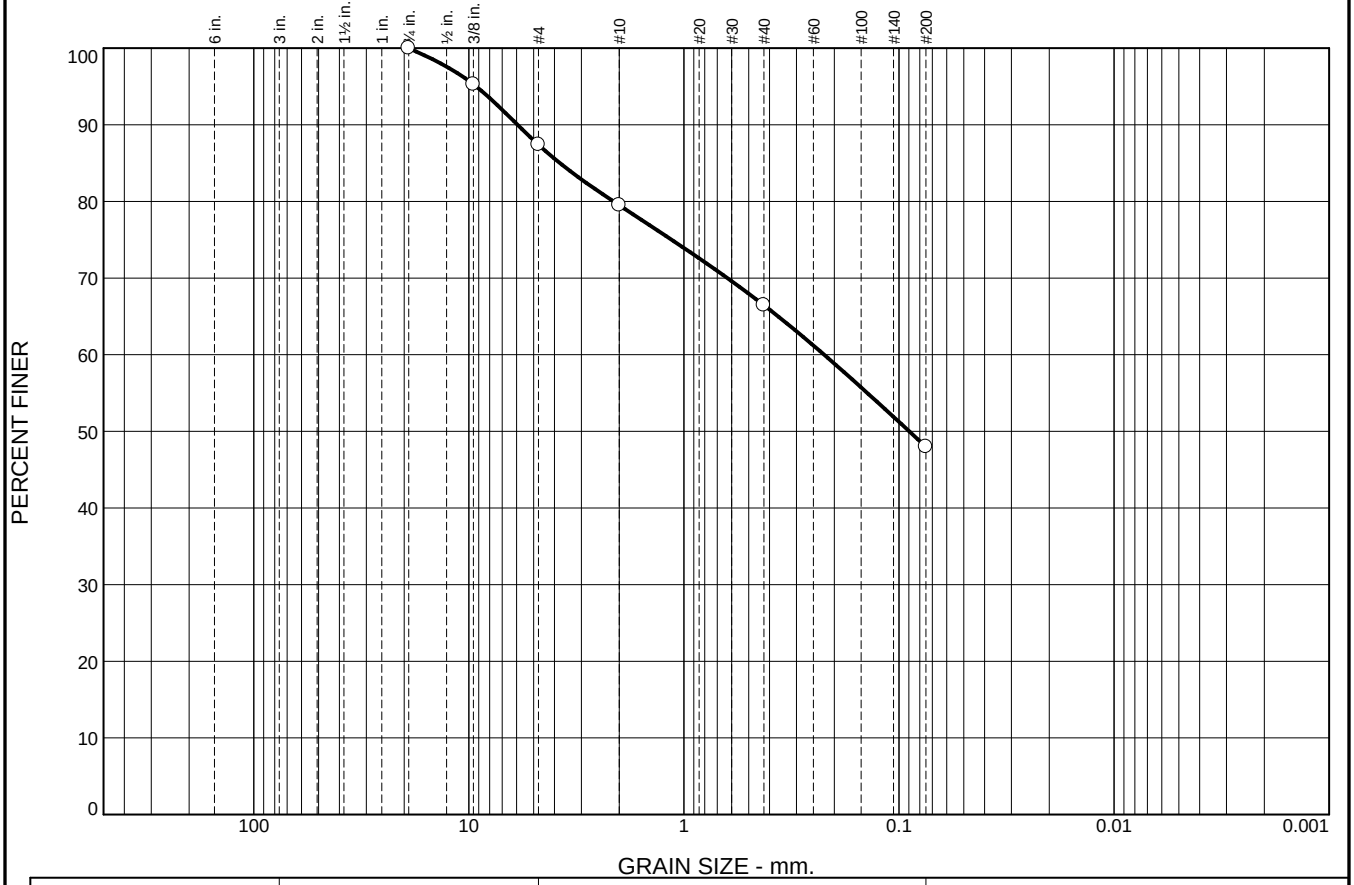
Figure 5

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	12.6	7.9	13.0	18.5	48.0	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			3.7714	0.2230	0.0896					

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SILTY SAND	05/24/21	SM	8.5

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-7 **Depth:** 6.0-8.0 FT **Sample Number:** S-4

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

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Figure 6

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report

	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	24.9	9.6	15.1	22.8	27.6	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			9.7063	1.1310	0.4104	0.0890				

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SILTY SAND WITH GRAVEL	05/24/21	SM	12.9

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-8 **Depth:** 6.0-8.0 FT **Sample Number:** S-4

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Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

Figure 7

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report

	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	9.1	6.5	11.5	19.5	53.4	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○	20	14	2.1735	0.1285						

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SANDY SILTY CLAY	05/28/21	CL-ML	12.0

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-9 TO B-14 **Sample Number:** BULK

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

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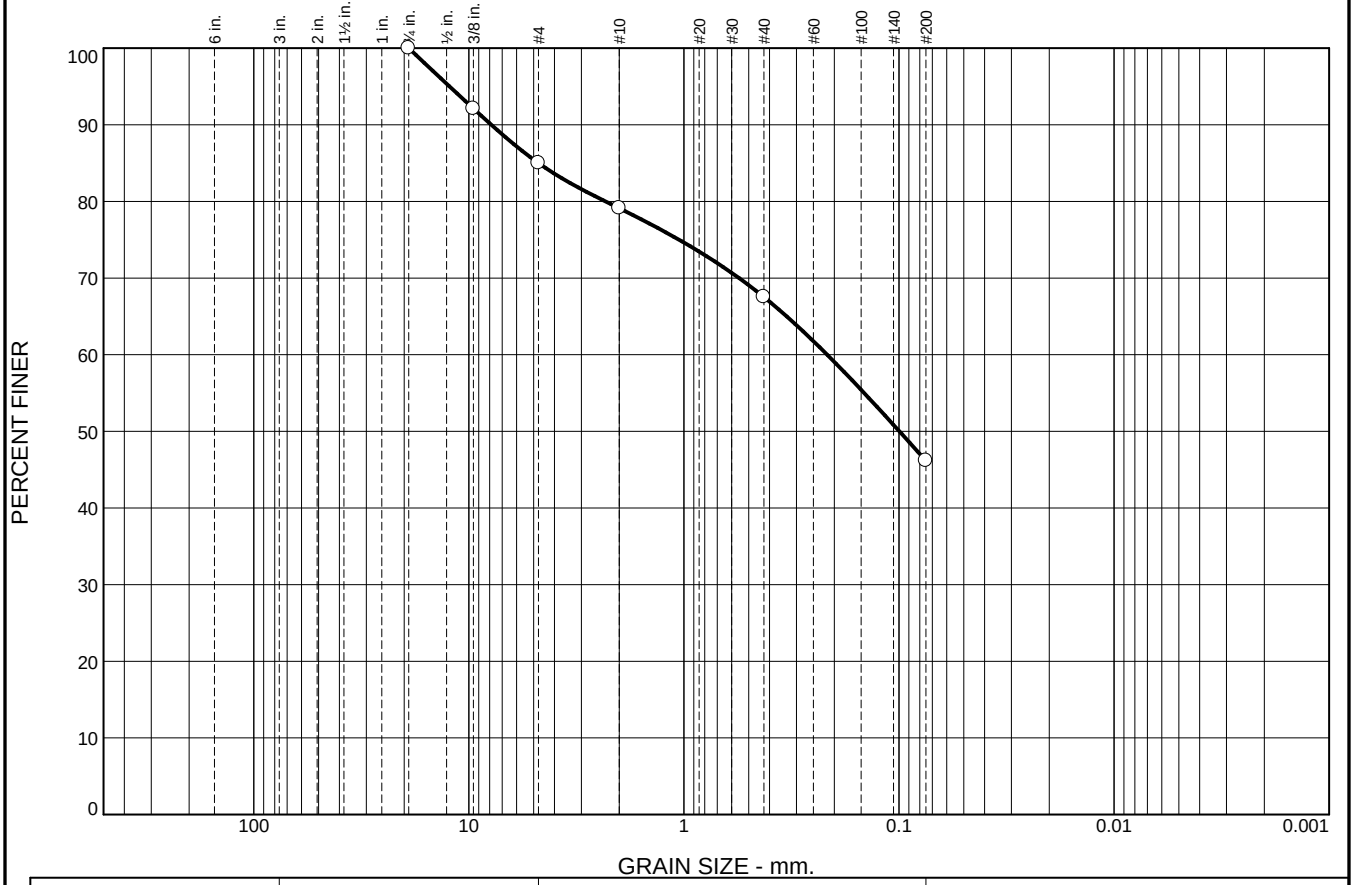
Figure 8

Tested By: CWZ 05/28/21 **Checked By:** JPB 06/04/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	15.0	5.9	11.6	21.3	46.2	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			4.7464	0.2157	0.0996					

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SILTY SAND WITH GRAVEL	05/24/21	SM	12.3

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-9 **Depth:** 6.0-8.0 FT **Sample Number:** S-4

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

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Figure 9

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report

	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	11.1	7.9	11.2	15.5	54.3	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			3.2965	0.1376						

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SANDY SILT	05/24/21	ML	10.6

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-12 **Depth:** 6.0-8.0 FT **Sample Number:** S-4

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

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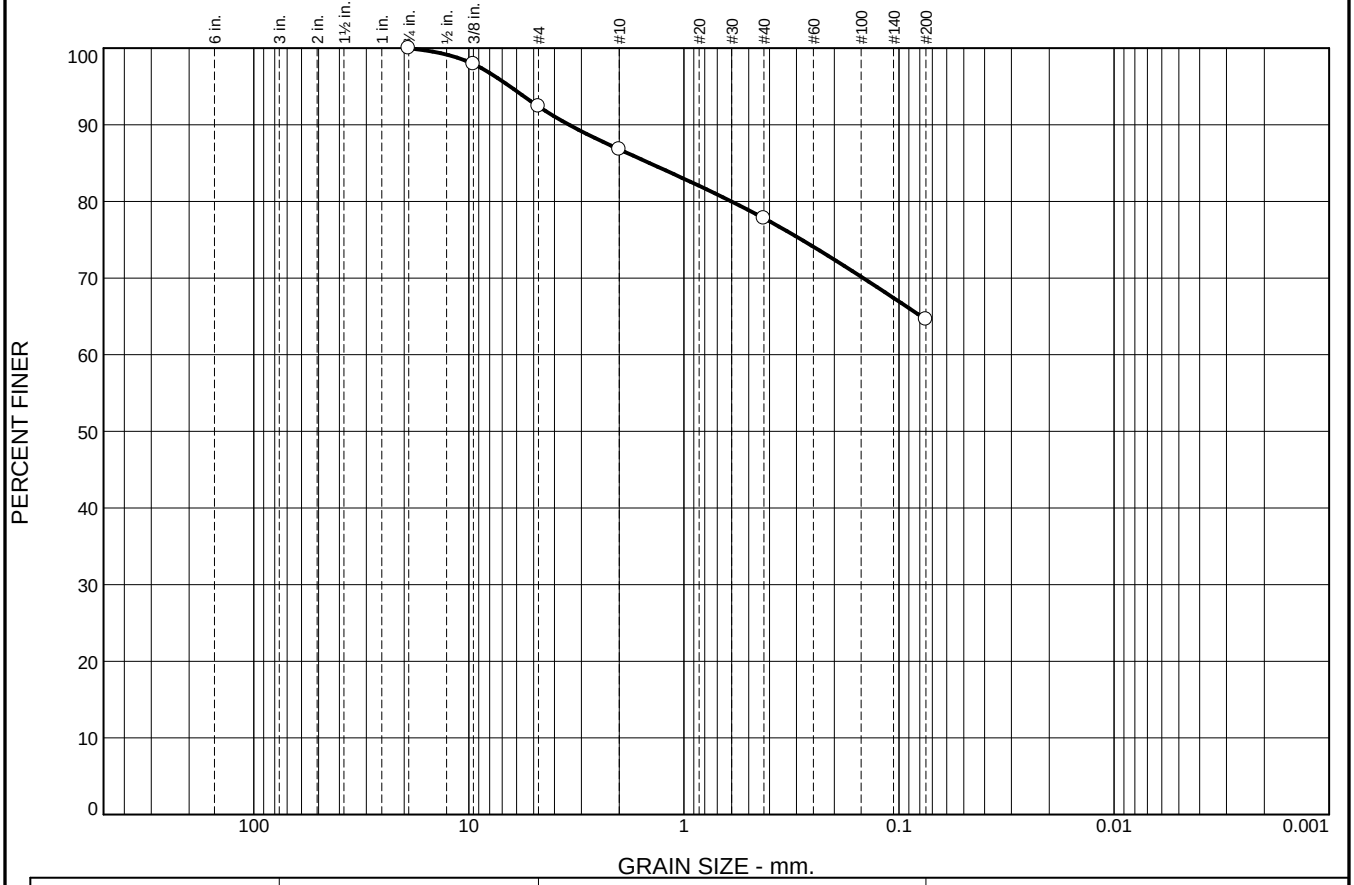
Figure 10

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	7.6	5.6	9.0	13.2	64.6	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			1.4451							

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SANDY SILT	05/24/21	ML	10.6

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-12 **Depth:** 13.0-15.0 FT **Sample Number:** S-6

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

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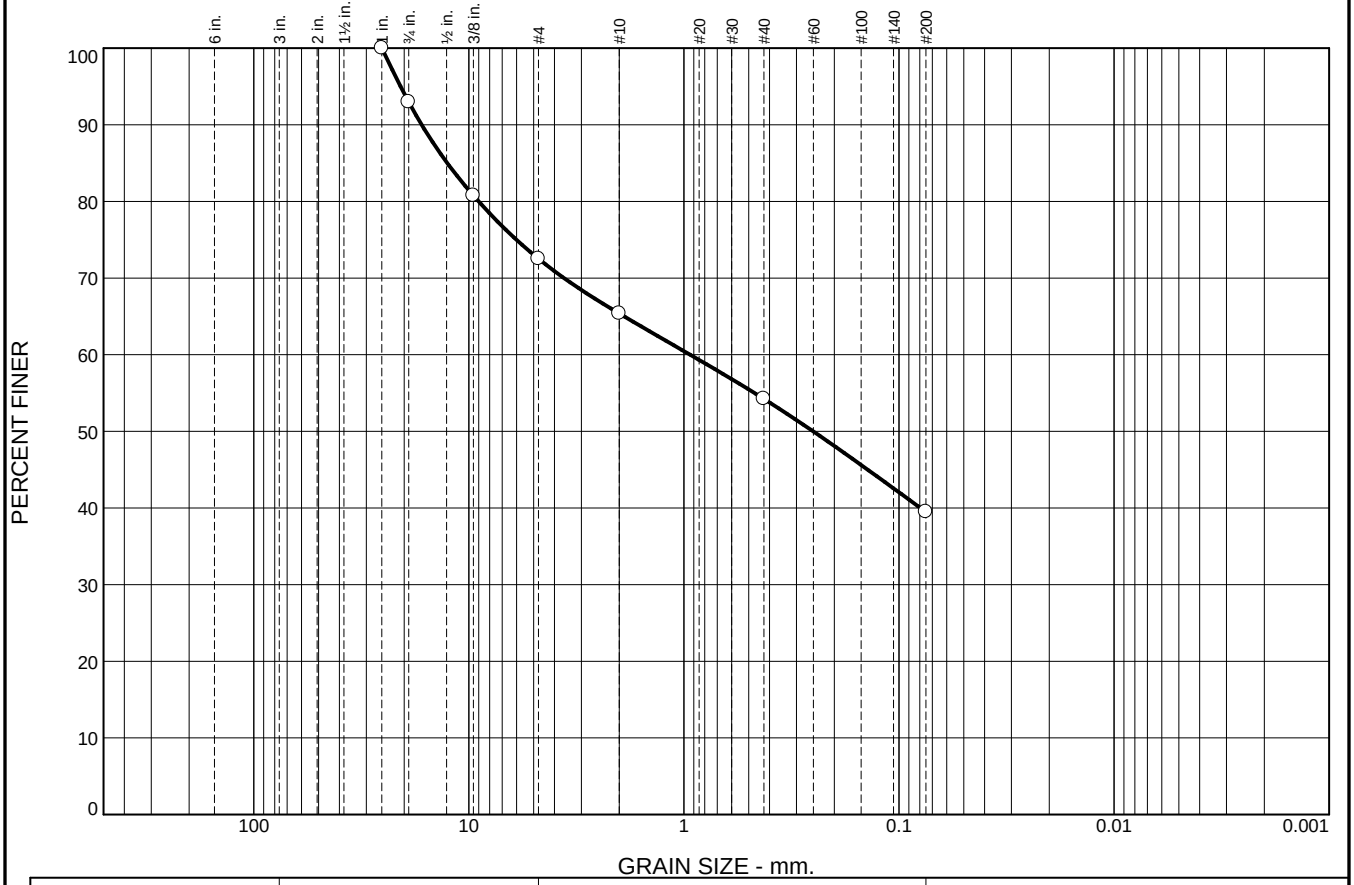
Figure 11

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	7.0	20.5	7.1	11.2	14.7	39.5	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			12.6024	0.9361	0.2507					

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ DARK BROWN TO BLACK SILTY SAND WITH GRAVEL	05/24/21	SM	6.6

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-15 **Depth:** 13.0-15.0 FT **Sample Number:** S-6

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

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Figure 12

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report

	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	7.1	15.0	6.0	10.6	13.9	47.4	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			8.7438	0.3574	0.1027					

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SILTY SAND WITH GRAVEL	05/24/21	SM	8.9

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-16 **Depth:** 8.0-10.0 FT **Sample Number:** S-5

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Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

Figure 13

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report

	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	12.8	2.7	5.3	9.9	14.7	54.6	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			5.2677	0.1379						

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SANDY SILT WITH GRAVEL	05/24/21	ML	22.7

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-21 **Depth:** 4.0-6.0 FT **Sample Number:** S-3

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

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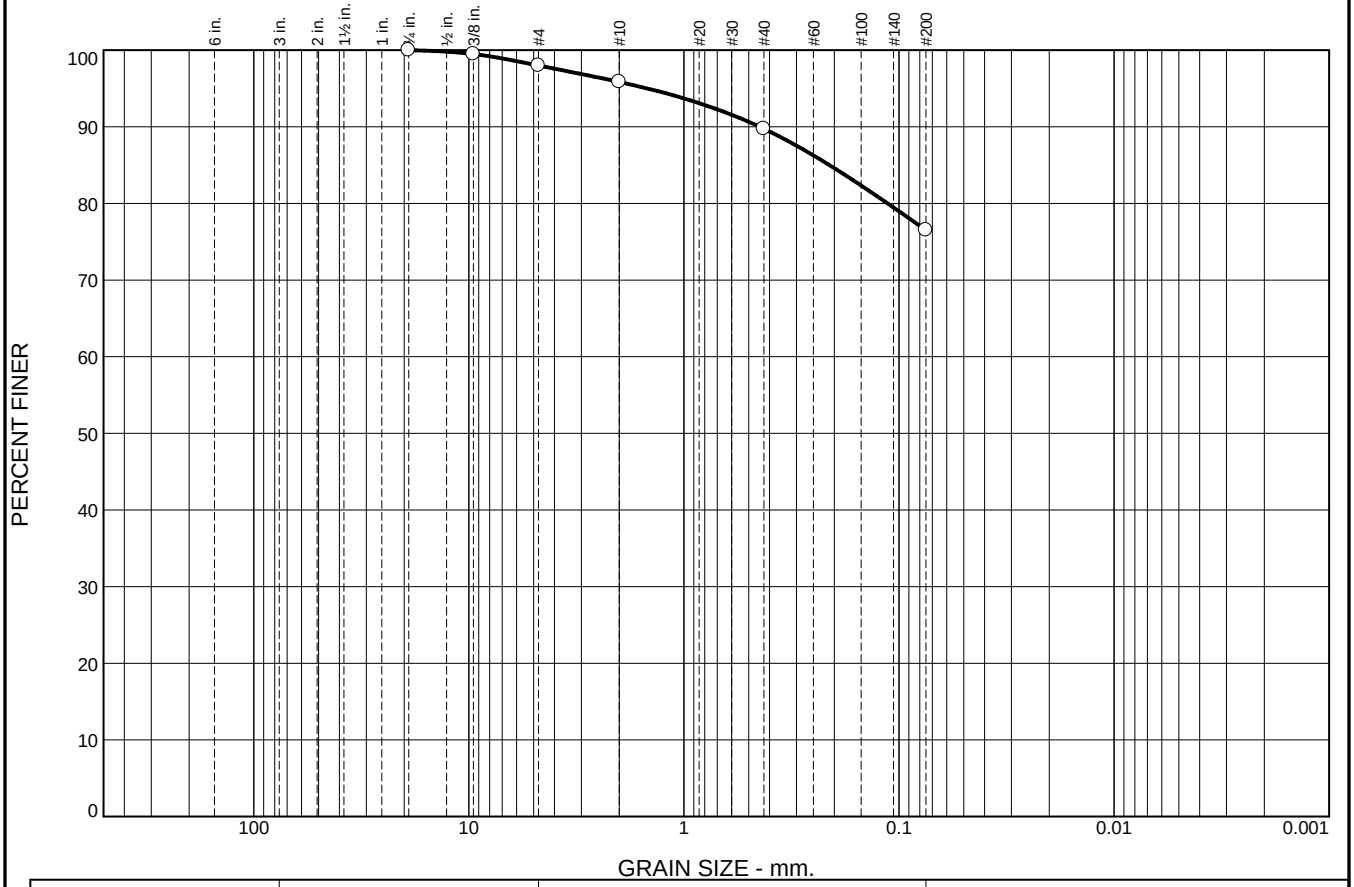
Figure 14

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	2.0	2.1	6.2	13.2	76.5	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○	18	14	0.2107							

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SILTY CLAY WITH SAND	05/28/21	CL-ML	19.7

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC

Project: MILL POINT SOLAR PROJECT

○ **Source:** B-22 TO B-29 **Depth:** 0.0-5.0 FT **Sample No.:** BULK 4

Remarks:

○SAMPLE DESCRIPTION BASED ON USCS

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Figure 15

Tested By: CWZ 05/28/21 **Checked By:** JPB 06/04/21

Particle Size Distribution Report

	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	1.9	1.6	8.6	20.9	67.0	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			0.3161							

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SANDY SILT	05/24/21	ML	31.4

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-25 **Depth:** 4.0-6.0 FT **Sample Number:** S-3

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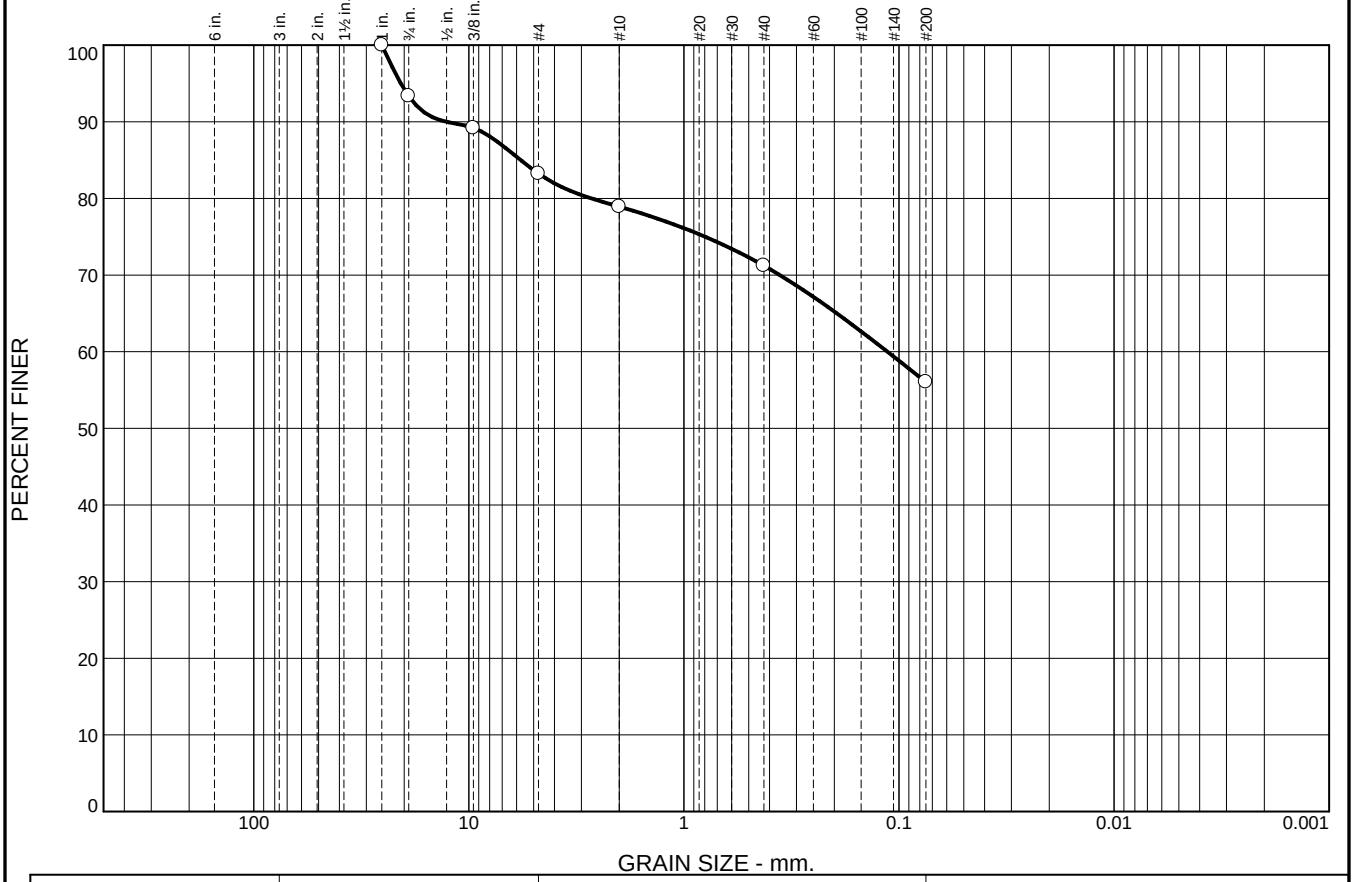
Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

Figure 16

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	6.6	10.2	4.3	7.7	15.2	56.0	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			5.7390	0.1132						

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SANDY SILT WITH GRAVEL	05/24/21	ML	17.9

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-27 **Depth:** 6.0-8.0 FT **Sample Number:** S-4

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

TRC Engineers, Inc.

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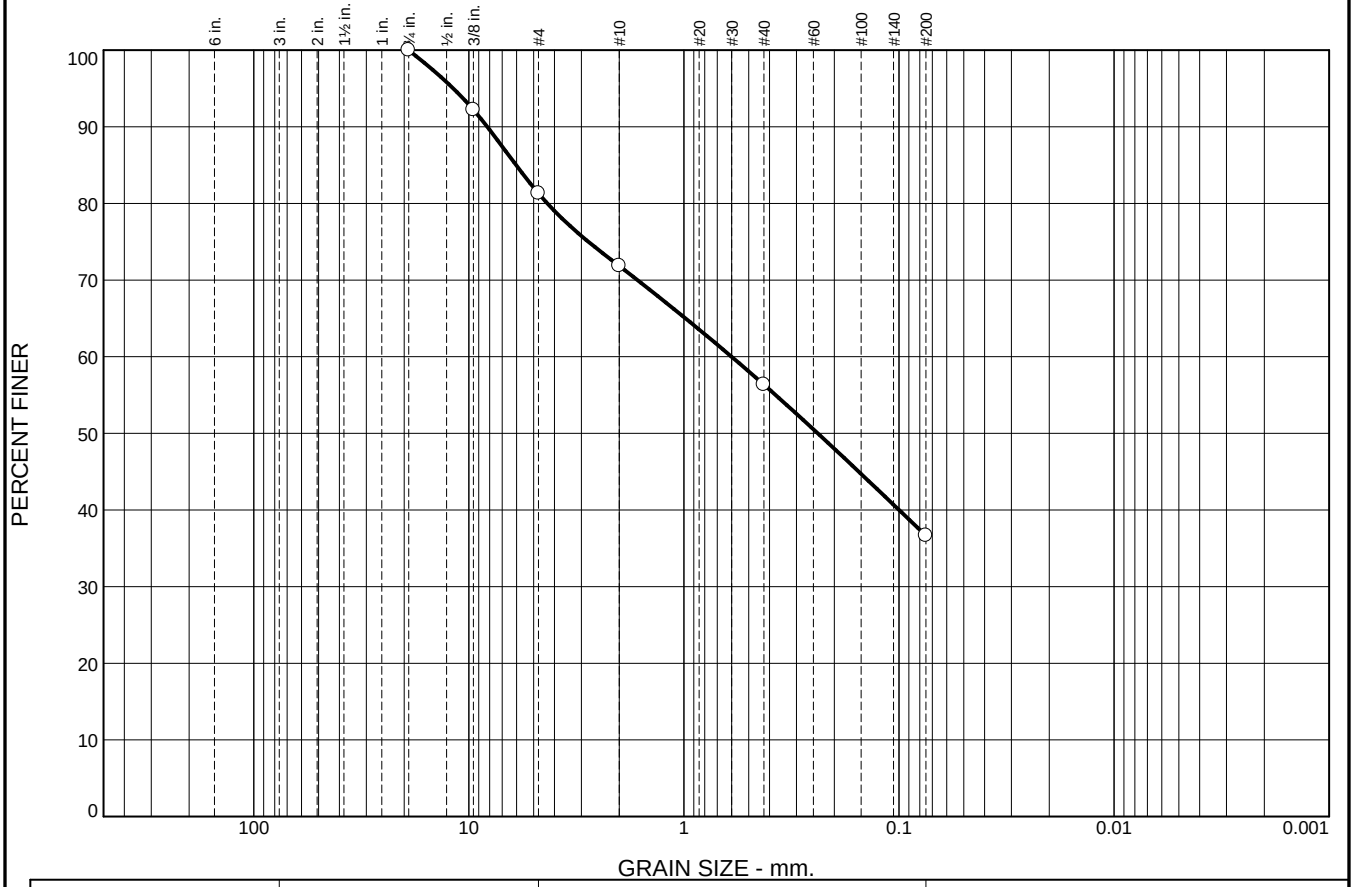
Figure 17

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	18.7	9.5	15.4	19.7	36.7	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			6.0299	0.6005	0.2386					

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ DARK BROWN TO BLACK SILTY SAND WITH GRAVEL	05/24/21	SM	5.4

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-29 **Depth:** 8.0-9.4 FT **Sample Number:** S-5

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

TRC Engineers, Inc.

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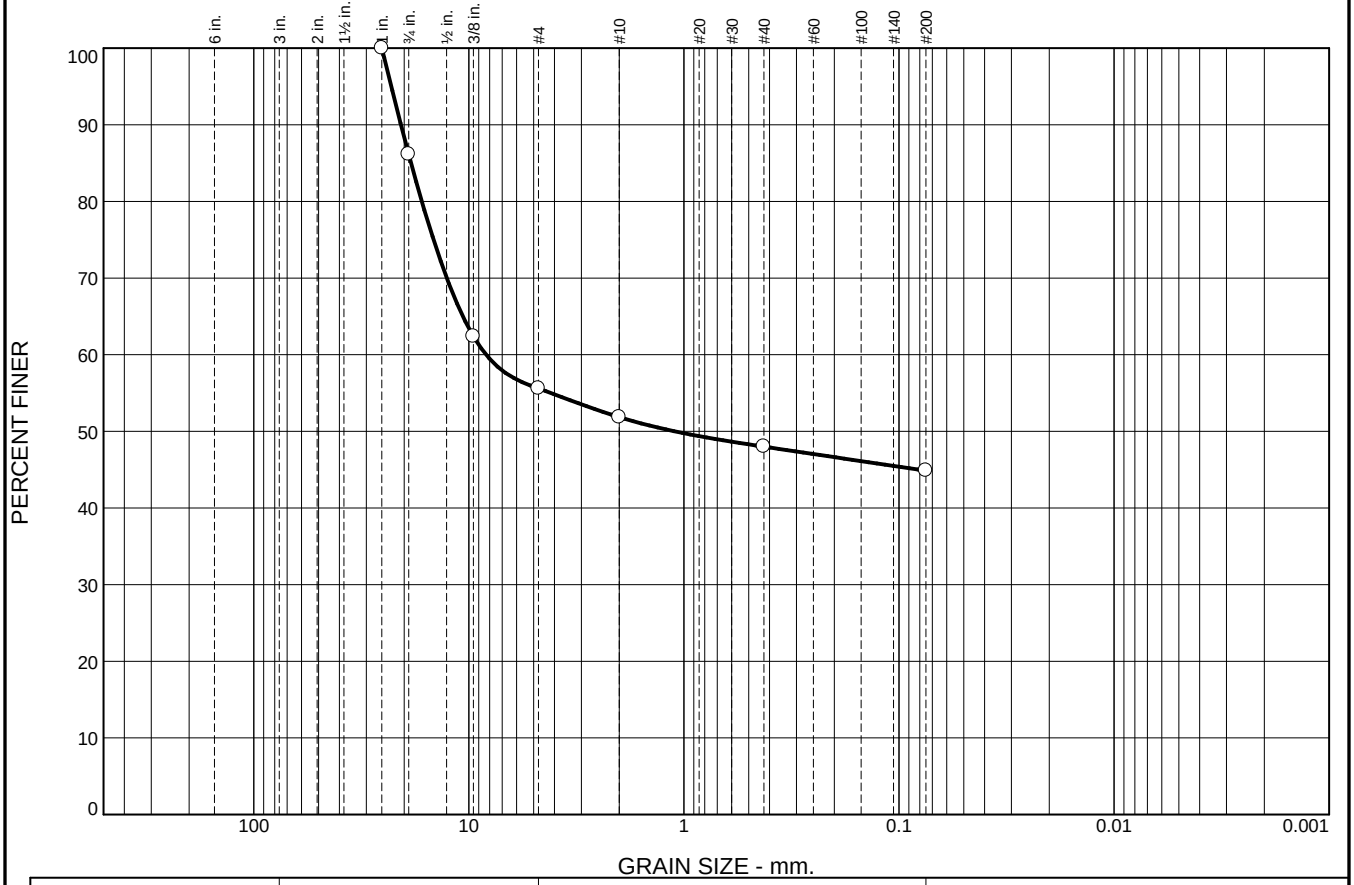
Figure 18

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	13.8	30.6	3.7	3.9	3.1	44.9	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			18.5696	8.3194	1.1026					

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SILTY GRAVEL	05/24/21	GM	31.9

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** B-30 **Depth:** 4.0-6.0 FT **Sample Number:** S-3

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

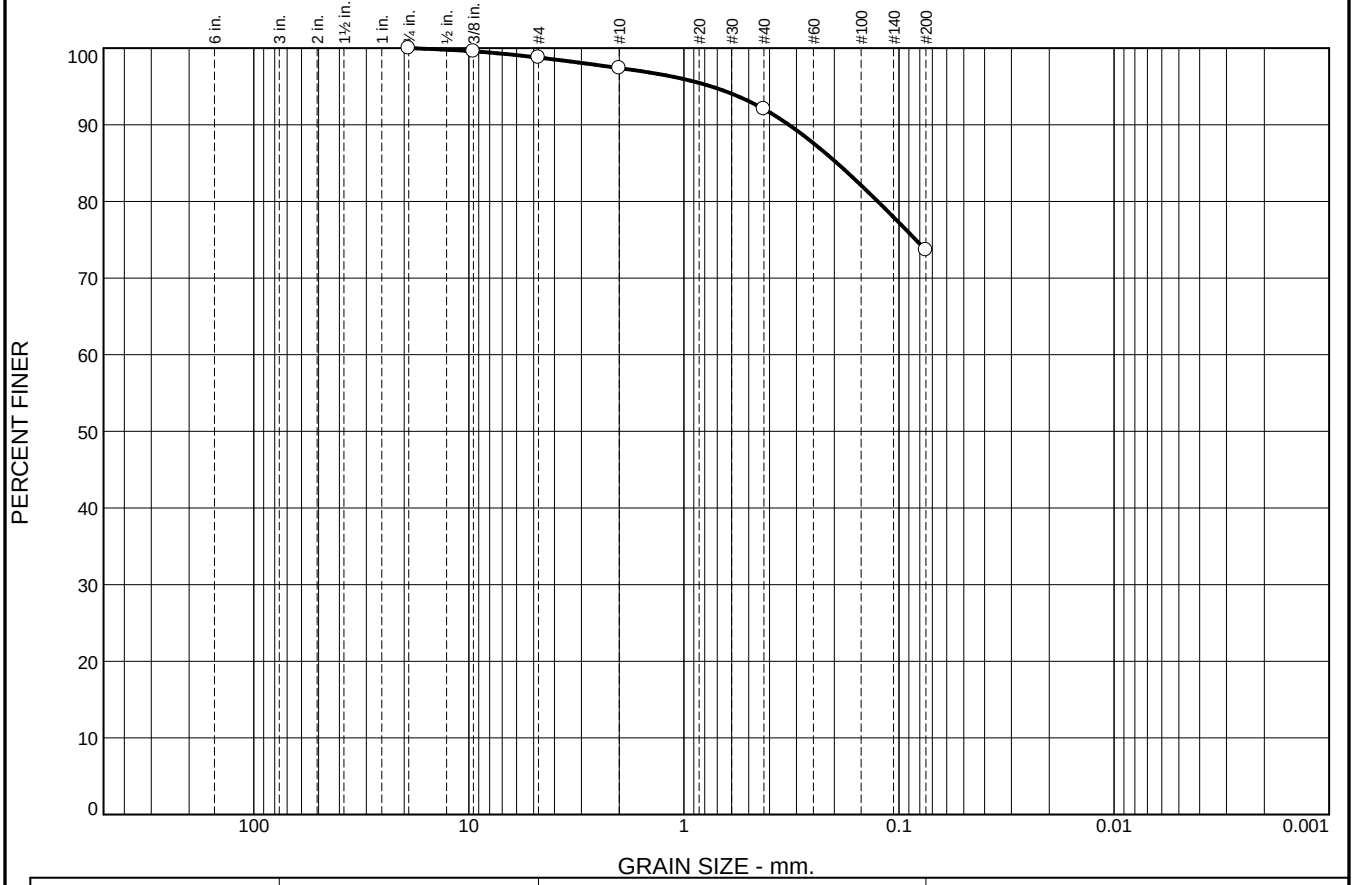
TRC Engineers, Inc.

Mt. Laurel, NJ

Figure 19

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	1.2	1.4	5.3	18.4	73.7	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○	43	25	0.1942							

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN CLAY WITH SAND	05/27/21	CL	40.1

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source:** SS-1 & SS-2 **Depth:** 0.0-5.0 FT **Sample No.:** BULK 5

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

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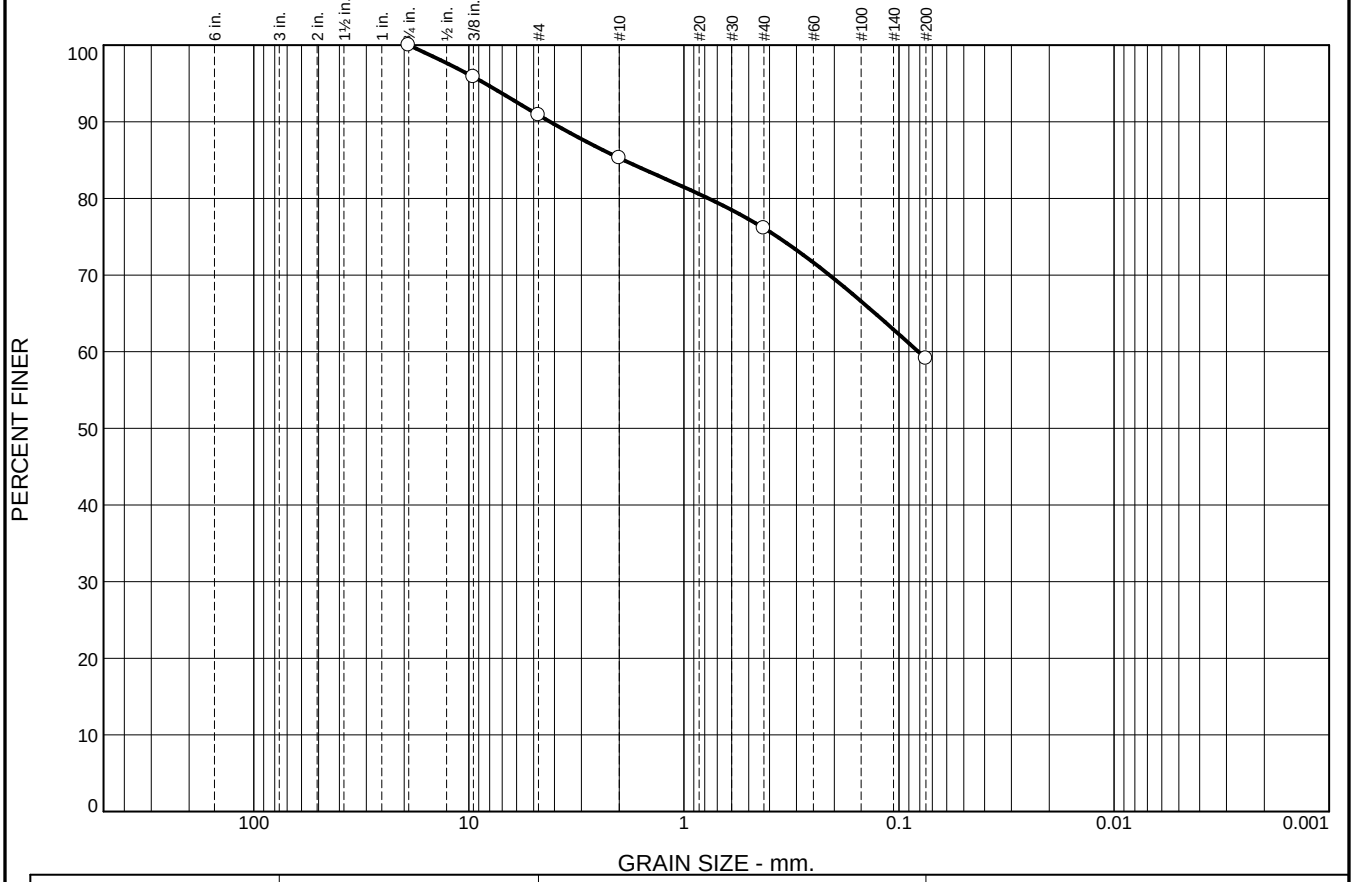
Figure 20

Tested By: CWZ 05/27/21 **Checked By:** JPB 06/04/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	9.1	5.6	9.2	17.0	59.1	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			1.9078	0.0813						

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ DARK BROWN SANDY SILT	05/24/21	ML	9.6

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** SS-1 **Depth:** 13.5-15.0 FT **Sample Number:** S-6

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

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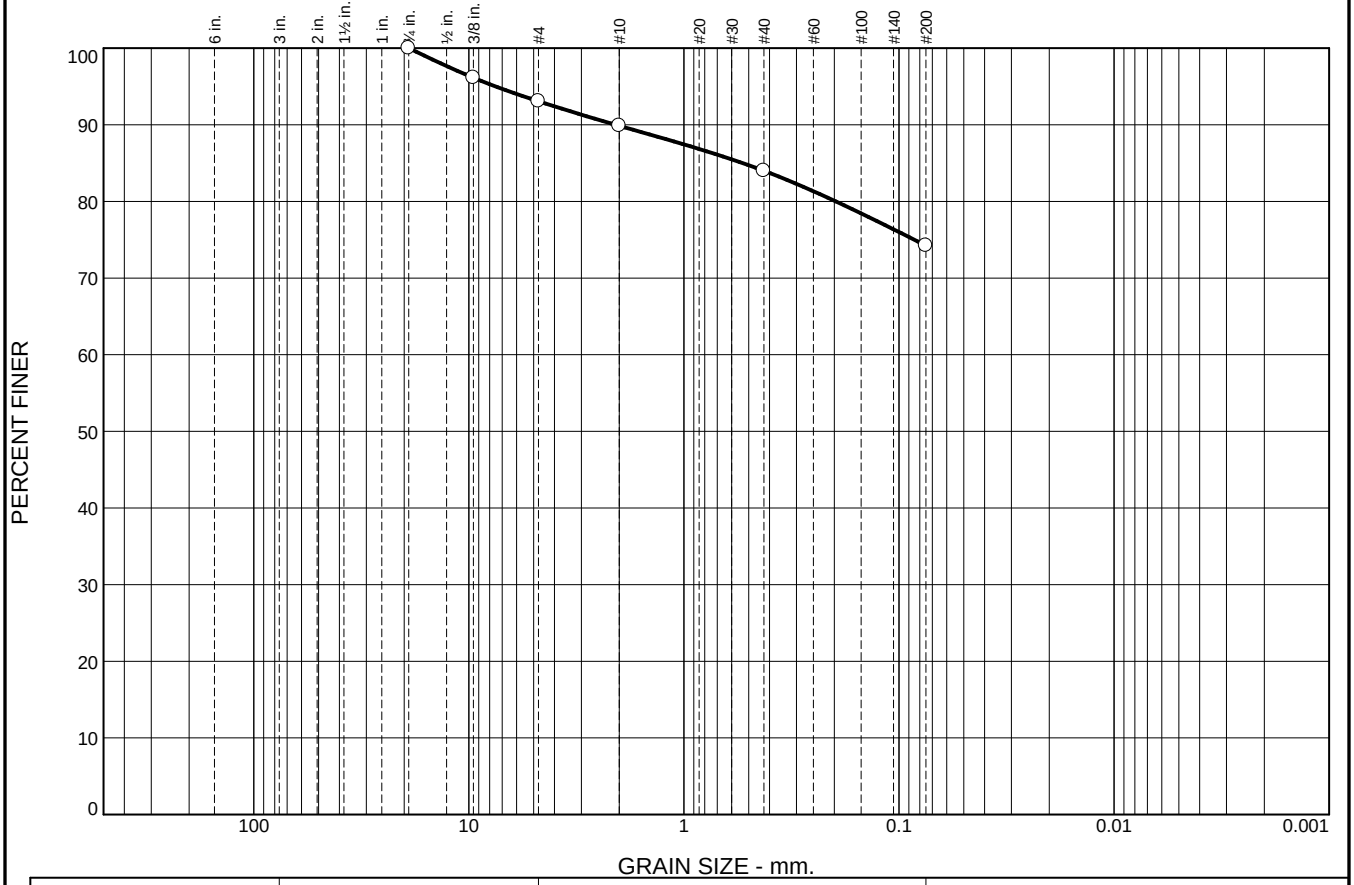
Figure 21

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report



	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	6.9	3.2	5.9	9.8	74.2	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			0.5328							

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ DARK BROWN SILTY CLAY WITH SAND	05/24/21	CL-ML	14.5

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** SS-1 **Depth:** 23.0-25.0 FT **Sample Number:** S-8

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

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Figure 22

Tested By: CWZ 05/27/21 **Checked By:** JPB 05/28/21

Particle Size Distribution Report

	% +3"	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	4.0	5.1	9.1	18.9	62.9	

	LL	PL	D_{85}	D_{60}	D_{50}	D_{30}	D_{15}	D_{10}	C_c	C_u
○			0.6686							

MATERIAL DESCRIPTION	TEST DATE	USCS	NM
○ BROWN SANDY SILT	05/24/21	ML	12.4

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○ **Source of Sample:** SS-2 **Depth:** 6.0-8.0 FT **Sample Number:** S-4

Remarks:
○SAMPLE DESCRIPTION BASED ON USCS

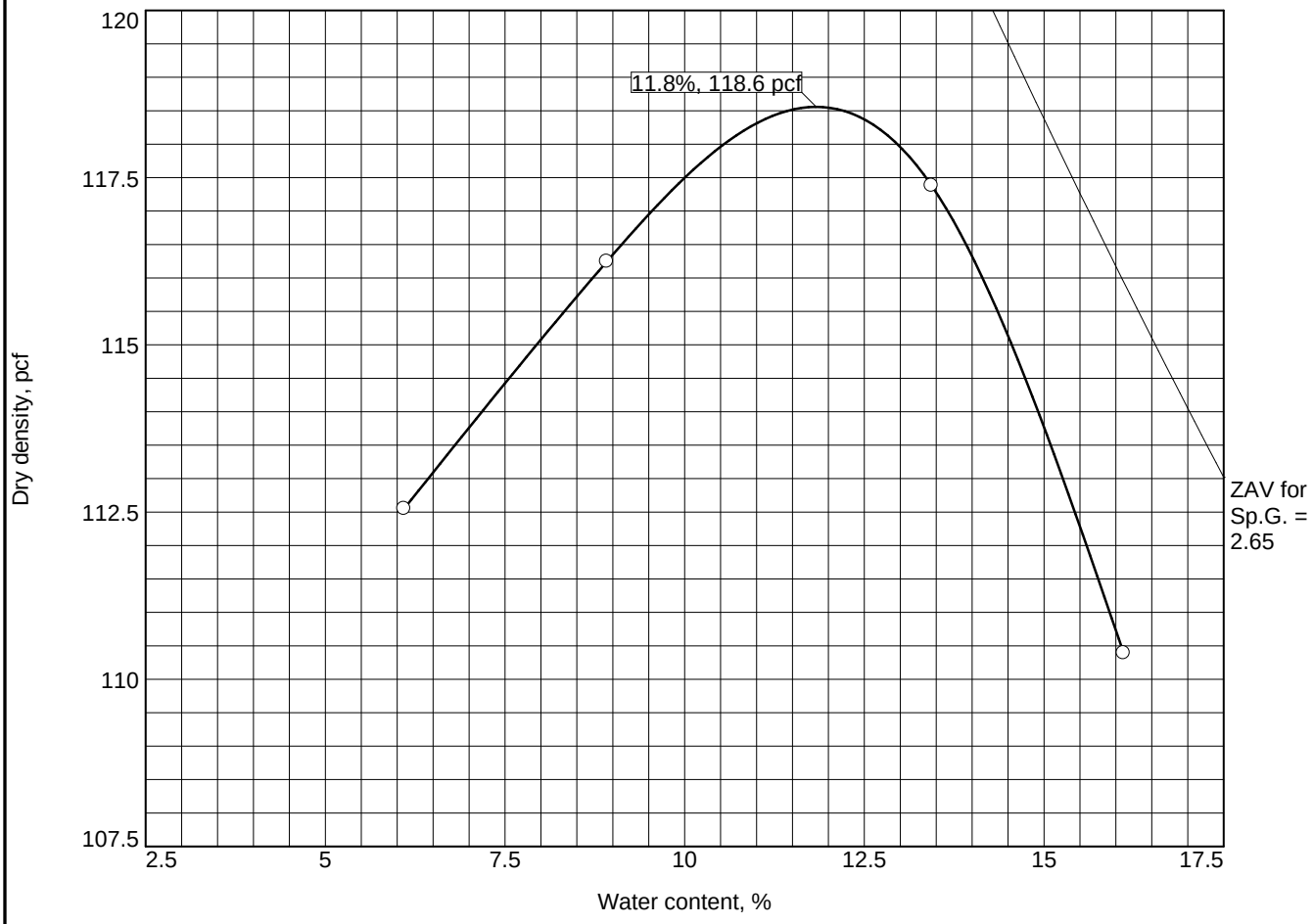
TRC Engineers, Inc.

Mt. Laurel, NJ

Figure 23

Tested By: CWZ 05/24/21 **Checked By:** JPB 05/28/21

COMPACTION TEST REPORT



Test specification: ASTM D 698-12 Method B Standard

Elev/ Depth	Classification		Nat. Moist.	Sp.G.	LL	PI	% > 3/8 in.	% < No.200
	USCS	AASHTO						
	CL-ML	A-4(0)	16.2		22	6	3.8	52.5

TEST RESULTS	MATERIAL DESCRIPTION
Maximum dry density = 118.6 pcf Optimum moisture = 11.8 %	BROWN SANDY SILTY CLAY

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○**Source of Sample:** B-1 TO B-5 **Sample Number:** BULK

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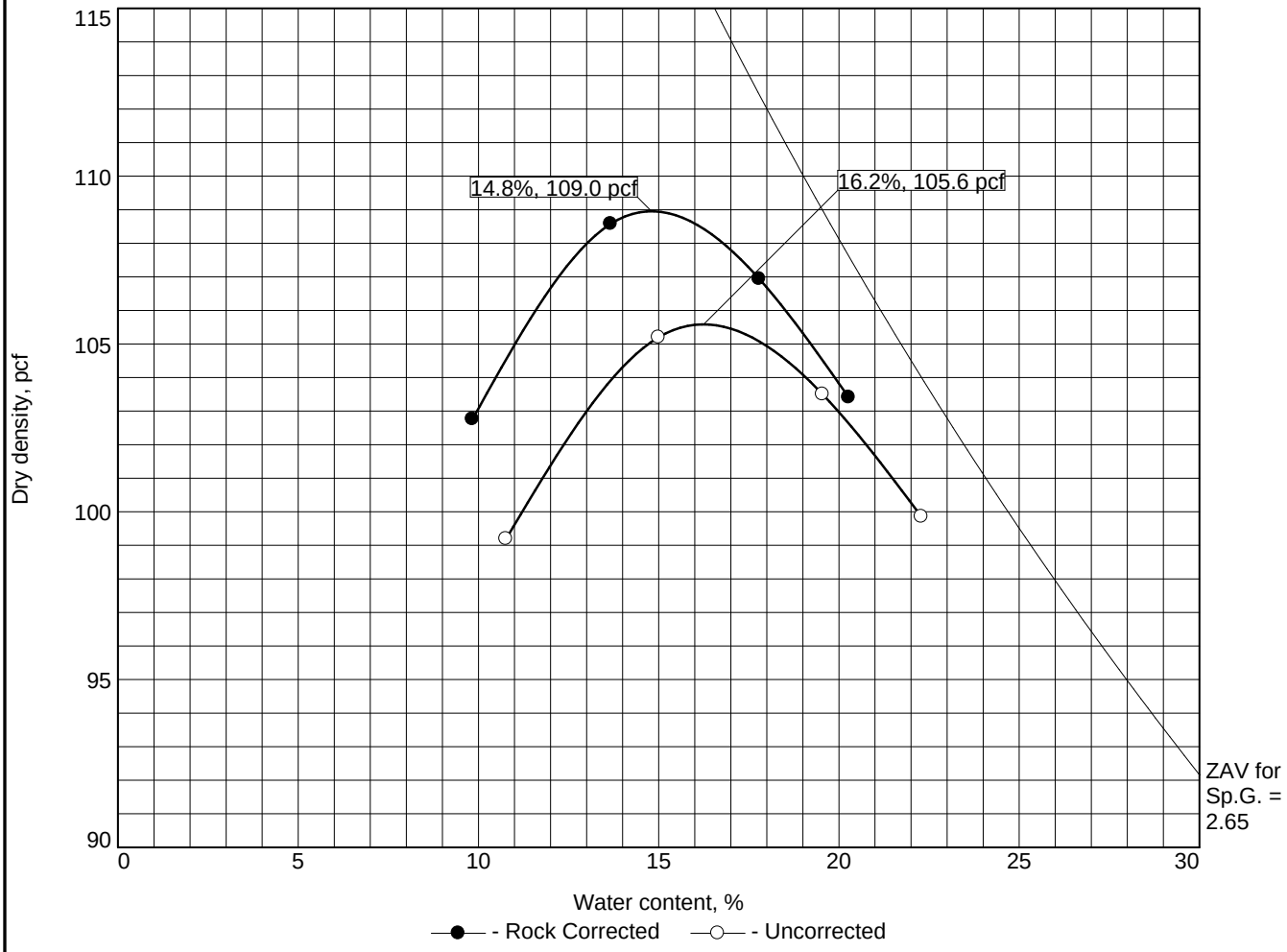
Remarks:
SAMPLE DESCRIPTION BASED ON USCS

Figure 24

Tested By: CWZ 05/17/21

COMPACTION TEST REPORT



Test specification: ASTM D 698-12 Method B Standard
ASTM D4718-15 Oversize Corr. Applied to Each Test Point

Elev/ Depth	Classification		Nat. Moist.	Sp.G.	LL	PI	% > 3/8 in.	% < No.200
	USCS	AASHTO						
	CL	A-6(4)	25.7		34	11	9.4	58.3

ROCK CORRECTED TEST RESULTS	UNCORRECTED	MATERIAL DESCRIPTION
Maximum dry density = 109.0 pcf	105.6 pcf	BROWN SANDY LEAN CLAY
Optimum moisture = 14.8 %	16.2 %	

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○**Source of Sample:** B-6 TO B-8 **Sample Number:** BULK

TRC Engineers, Inc.

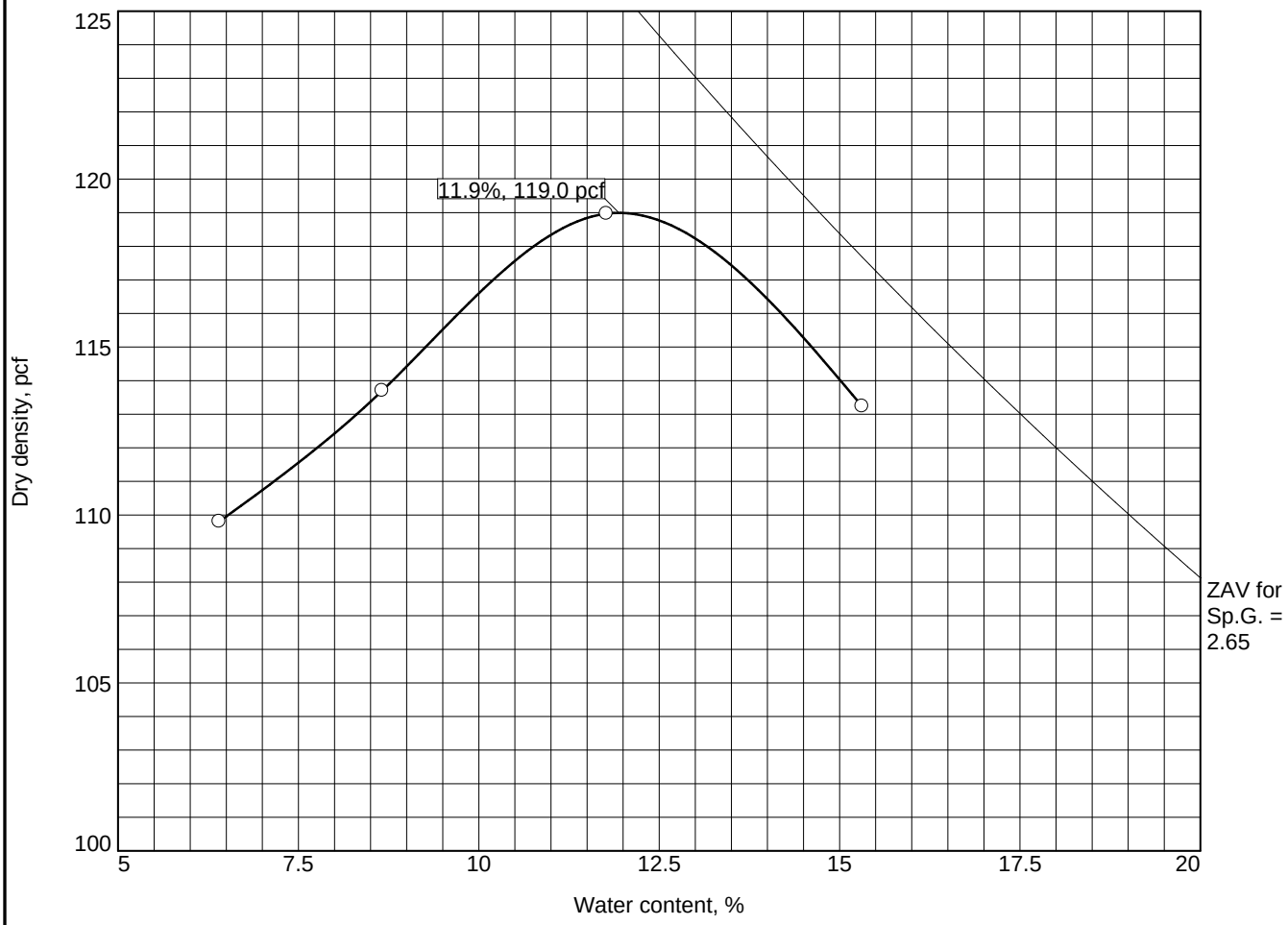
Mt. Laurel, NJ

Remarks:
SAMPLE DESCRIPTION BASED ON USCS

Figure 25

Tested By: CWZ 05/17/21

COMPACTION TEST REPORT



Test specification: ASTM D 698-12 Method B Standard

Elev/ Depth	Classification		Nat. Moist.	Sp.G.	LL	PI	% > 3/8 in.	% < No.200
	USCS	AASHTO						
	CL-ML	A-4(0)	12.0		20	6	4.3	53.4

TEST RESULTS	MATERIAL DESCRIPTION
Maximum dry density = 119.0 pcf Optimum moisture = 11.9 %	BROWN SANDY SILTY CLAY

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○**Source of Sample:** B-9 TO B-14 **Sample Number:** BULK

TRC Engineers, Inc.

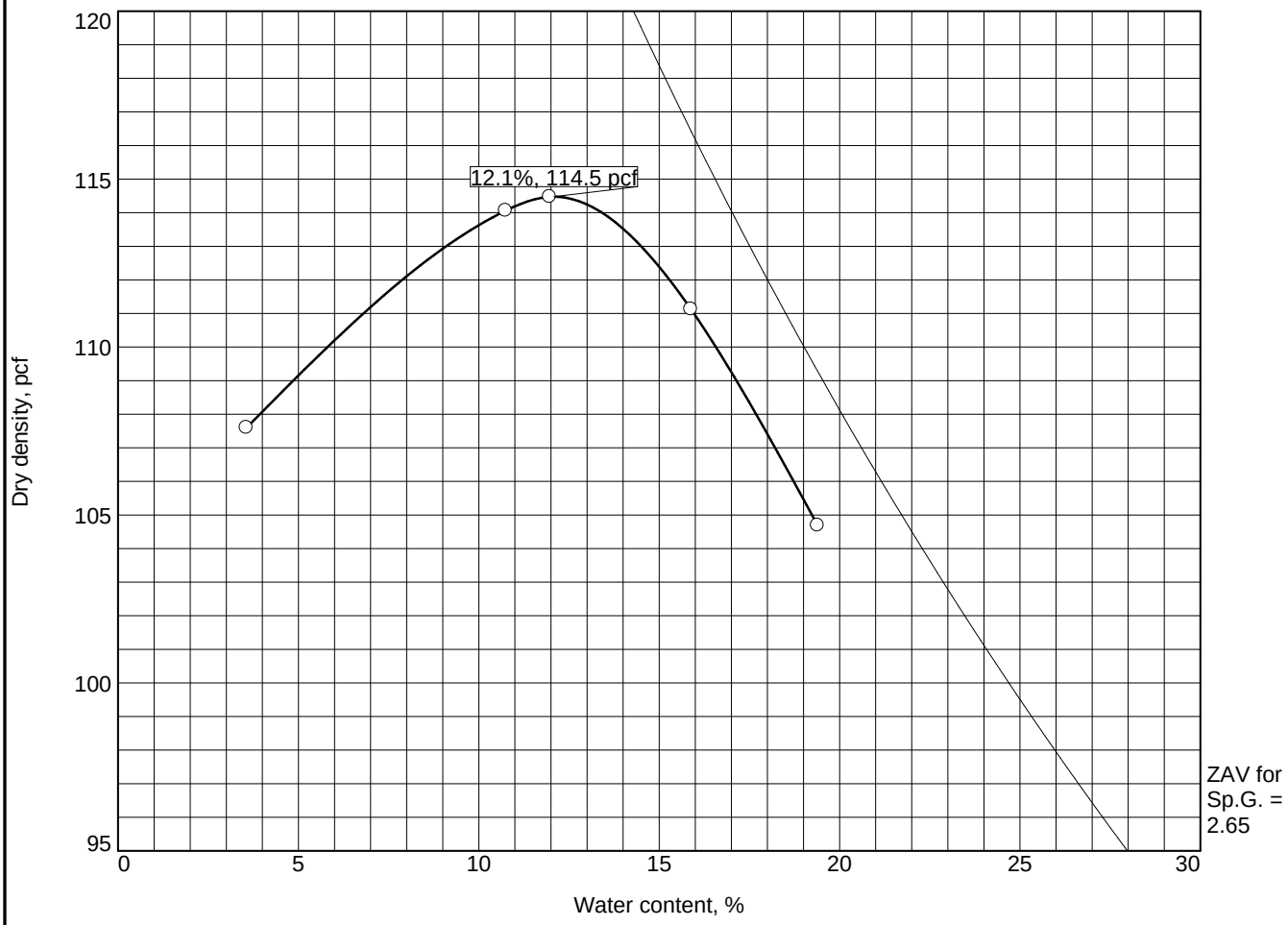
Mt. Laurel, NJ

Remarks:
SAMPLE DESCRIPTION BASED ON USCS

Figure 26

Tested By: CWZ 05/17/21

COMPACTION TEST REPORT



Test specification: ASTM D 698-12 Method B Standard

Elev/ Depth	Classification		Nat. Moist.	Sp.G.	LL	PI	% > 3/8 in.	% < No.200
	USCS	AASHTO						
0.0-5.0 FT	CL-ML	A-4(0)	19.7		18	4	0.5	76.5

TEST RESULTS	MATERIAL DESCRIPTION
Maximum dry density = 114.5 pcf Optimum moisture = 12.1 %	BROWN SILTY CLAY WITH SAND

Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC
Project: MILL POINT SOLAR PROJECT

○**Source of Sample:** B-22 TO B-29 **Sample Number:** BULK 4

TRC Engineers, Inc.

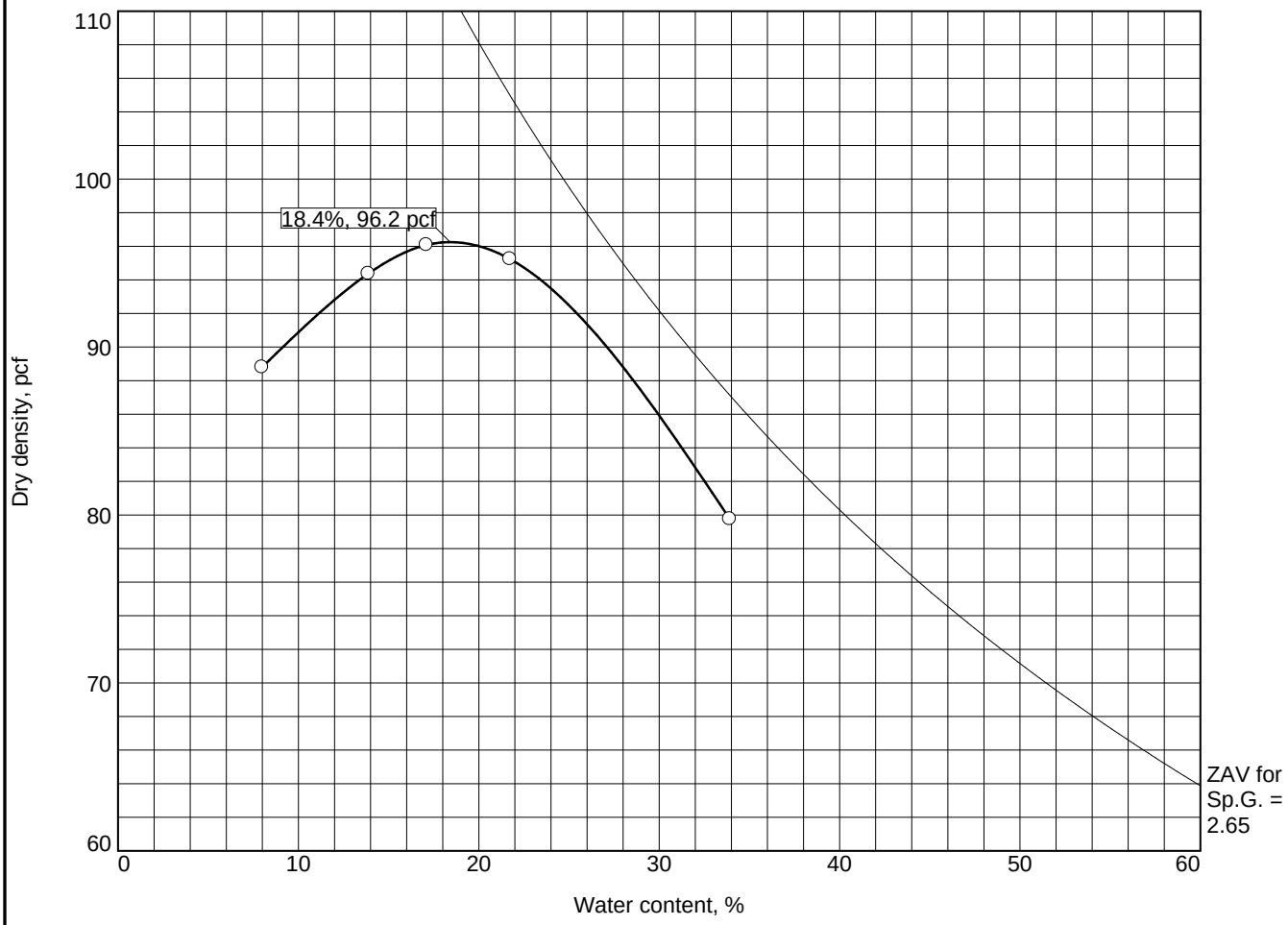
Mt. Laurel, NJ

Remarks:
SAMPLE DESCRIPTION BASED ON USCS

Figure 27

Tested By: WM 05/25/21 **Checked By:** JPB 05/28/21

COMPACTION TEST REPORT



Test specification: ASTM D 698-12 Method B Standard

Elev/ Depth	Classification		Nat. Moist.	Sp.G.	LL	PI	% > 3/8 in.	% < No.200
	USCS	AASHTO						
0.0-5.0 FT	CL	A-7-6(13)	40.1		43	18	0.4	73.7

TEST RESULTS	MATERIAL DESCRIPTION
Maximum dry density = 96.2 pcf Optimum moisture = 18.4 %	BROWN CLAY WITH SAND
Project No. 411360.GEO1 **Client:** CONNECTGEN, LLC **Project:** MILL POINT SOLAR PROJECT ○**Source of Sample:** SS-1 & SS-2 **Sample Number:** BULK 5 **TRC Engineers, Inc.** **Mt. Laurel, NJ**	**Remarks:** SAMPLE DESCRIPTION BASED ON USCS **Figure** 28

Tested By: WM 05/25/21 **Checked By:** JPB 05/28/21

TRC ENGINEERS, INC.
UNIT WEIGHT

Job # **411360.GEO1**
Boring No. **B-2**
Sample No. **S-5**
Lift #: **8.0-10.0**

Job Name: **Mill Point Solar Project**
Client Name: **ConnectGen, LLC**

Height 1.9496
Diameter 1.3584

	g
Moist Sample Weight + Tare (g)	227.03
Dry Sample Weight + Tare (g)	218.21
Tare weight (g)	125.5

	g	lbs
Moist Sample Weight - Tare	101.53	0.223634
Dry Sample Weight - Tare	92.71	0.204207
Weight of Water	8.82	-
Moisture %	9.5	-
Moisture Content	0.095	-

Sample Total Area 1.45 in2

Total Volume (cu in) 2.82

Total Volume (cu ft) 0.0016342

Dry Unit Weight (pcf) **125.0**

Wet Unit Weight (pcf) **136.8**

TRC ENGINEERS, INC.
UNIT WEIGHT

Job # **411360.GEO1**
Boring No. **B-12**
Sample No. **S-4**
Lift #: **6.0-8.0**

Job Name: **Mill Point Solar Project**
Client Name: **ConnectGen, LLC**

Height 1.2361
Diameter 1.3703

	g
Moist Sample Weight + Tare (g)	228.37
Dry Sample Weight + Tare (g)	222.06
Tare weight (g)	162.34

	g	lbs
Moist Sample Weight - Tare	66.03	0.145441
Dry Sample Weight - Tare	59.72	0.131542
Weight of Water	6.31	-
Moisture %	10.6	-
Moisture Content	0.106	-

Sample Total Area 1.47 in2

Total Volume (cu in) 1.82

Total Volume (cu ft) 0.0010544

Dry Unit Weight (pcf) **124.8**

Wet Unit Weight (pcf) **137.9**

TRC ENGINEERS, INC.
UNIT WEIGHT

Job # **411360.GEO1**
Boring No. **B-12**
Sample No. **S-6**
Lift #: **13.0-15.0**

Job Name: **Mill Point Solar Project**
Client Name: **ConnectGen, LLC**

Height 1.8373
Diameter 1.3748

	g
Moist Sample Weight + Tare (g)	255.53
Dry Sample Weight + Tare (g)	246.16
Tare weight (g)	157.65

	g	lbs
Moist Sample Weight - Tare	97.88	0.215595
Dry Sample Weight - Tare	88.51	0.194956
Weight of Water	9.37	-
Moisture %	10.6	-
Moisture Content	0.106	-

Sample Total Area 1.48 in2

Total Volume (cu in) 2.73

Total Volume (cu ft) 0.0015774

Dry Unit Weight (pcf) **123.6**

Wet Unit Weight (pcf) **136.7**

TRC ENGINEERS, INC.
UNIT WEIGHT

Job # **411360.GEO1**
Boring No. **B-19**
Sample No. **S-2**
Lift #: **2.0-4.0**

Job Name: **Mill Point Solar Project**
Client Name: **ConnectGen, LLC**

Height 2.7996
Diameter 1.3778

	g
Moist Sample Weight + Tare (g)	267.71
Dry Sample Weight + Tare (g)	239.02
Tare weight (g)	136.1

	g	lbs
Moist Sample Weight - Tare	131.61	0.289890
Dry Sample Weight - Tare	102.92	0.226696
Weight of Water	28.69	-
Moisture %	27.9	-
Moisture Content	0.279	-

Sample Total Area 1.49 in2

Total Volume (cu in) 4.17

Total Volume (cu ft) 0.0024142

Dry Unit Weight (pcf) **93.9**

Wet Unit Weight (pcf) **120.1**

TRC ENGINEERS, INC.
UNIT WEIGHT

Job # **411360.GEO1**
Boring No. **B-21**
Sample No. **S-5**
Lift #: **8.0-10.0**

Job Name: **Mill Point Solar Project**
Client Name: **ConnectGen, LLC**

Height 2.8235
Diameter 1.4788

	g
Moist Sample Weight + Tare (g)	284.85
Dry Sample Weight + Tare (g)	249.52
Tare weight (g)	128.36

	g	lbs
Moist Sample Weight - Tare	156.49	0.344692
Dry Sample Weight - Tare	121.16	0.266872
Weight of Water	35.33	-
Moisture %	29.2	-
Moisture Content	0.292	-

Sample Total Area 1.72 in2

Total Volume (cu in) 4.85

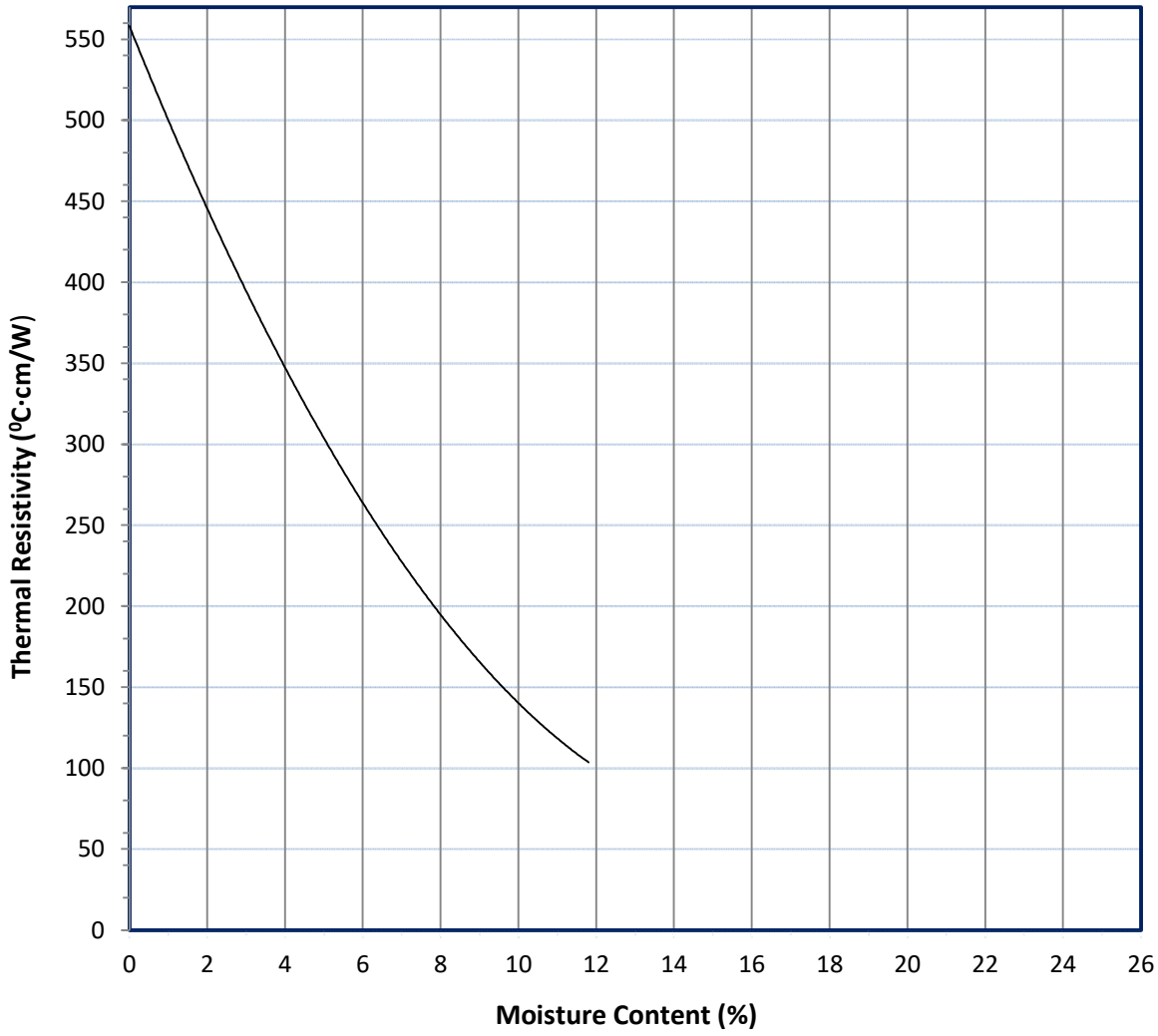
Total Volume (cu ft) 0.0028048

Dry Unit Weight (pcf) **95.1**

Wet Unit Weight (pcf) **122.9**

B-1 to B-5, Bulk 1, 0.0-5.0 ft

THERMAL RESISTIVITY DRY-OUT CURVES
411360.GEO1: Mill Point Solar Project

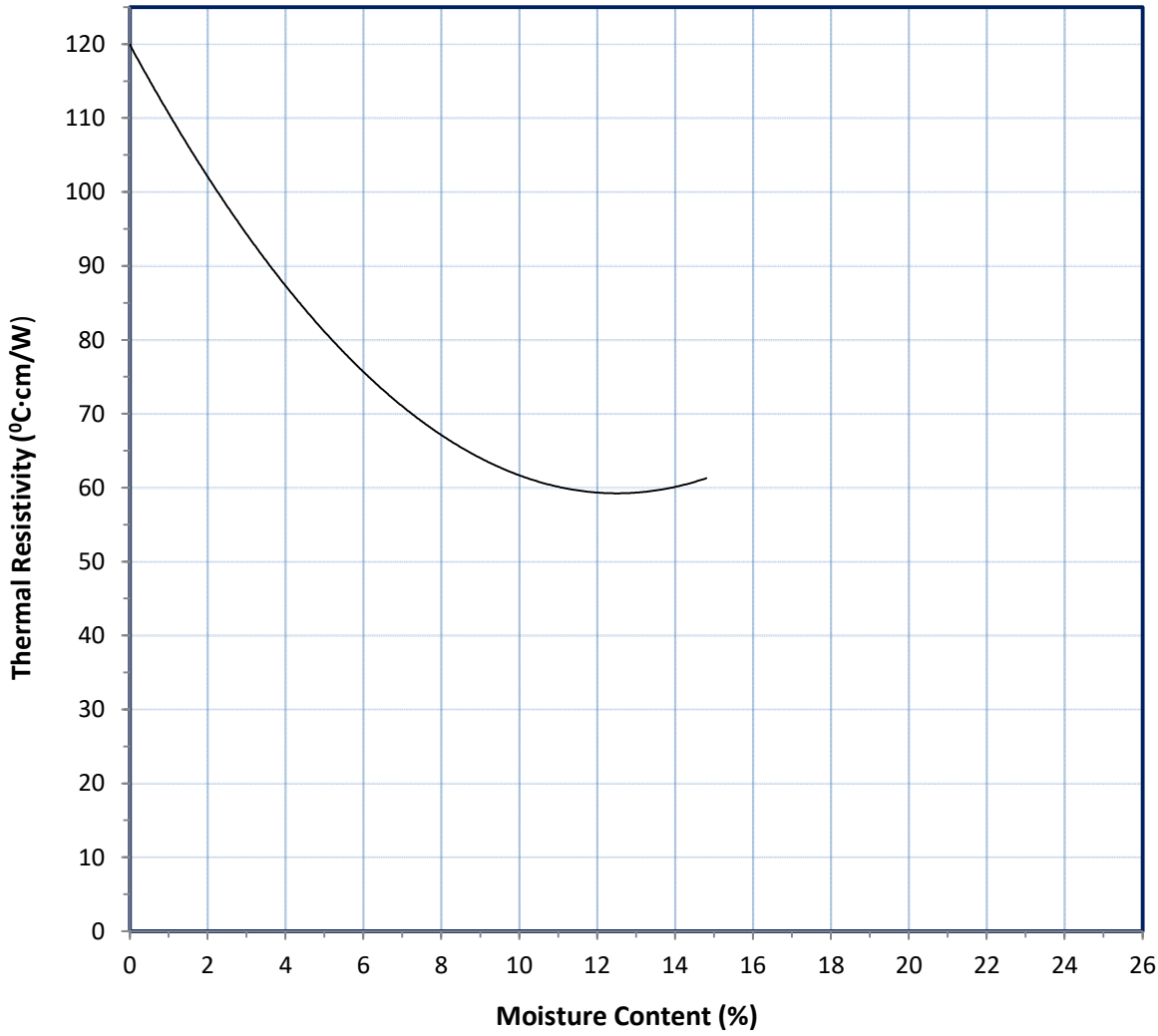


Specimen ID:	B-1 to B-5, Bulk 1, 0.0-5.0 ft
USCS:	CL-ML
Received Moisture:	16.2%
LL:	22
PI:	6
P200:	52.5%
Max. Dry Dens.:	118.6 pcf
Optimum Moisture:	11.8%

Specimen was prepared at the optimum moisture content and at approximately 90% of the maximum dry density as determined by the Standard Proctor test.

B-6 to B-8, Bulk 2, 0.0-5.0 ft

THERMAL RESISTIVITY DRY-OUT CURVES
411360.GEO1: Mill Point Solar Project

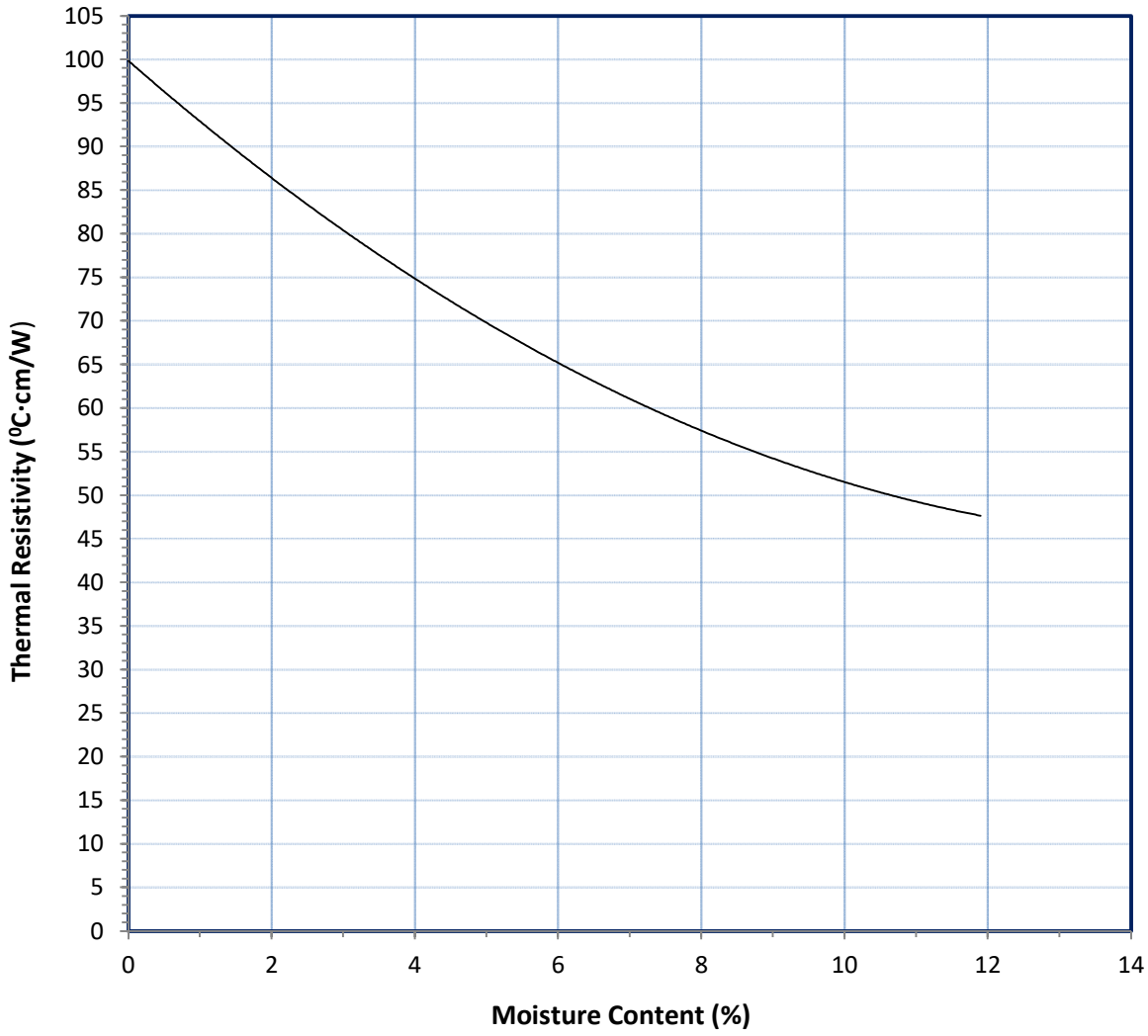


Specimen ID:	B-6 to B-8, Bulk 2, 0.0-5.0 ft
USCS:	CL
Received Moisture:	25.7%
LL:	34
PI:	11
P200:	58.3%
Max. Dry Dens.:	109.0 pcf
Optimum Moisture:	14.8%

Specimen was prepared at the optimum moisture content and at approximately 90% of the maximum dry density as determined by the Standard Proctor test.

B-9 to B-14, Bulk 3, 0.0-5.0 ft

THERMAL RESISTIVITY DRY-OUT CURVES
411360.GEO1: Mill Point Solar Project

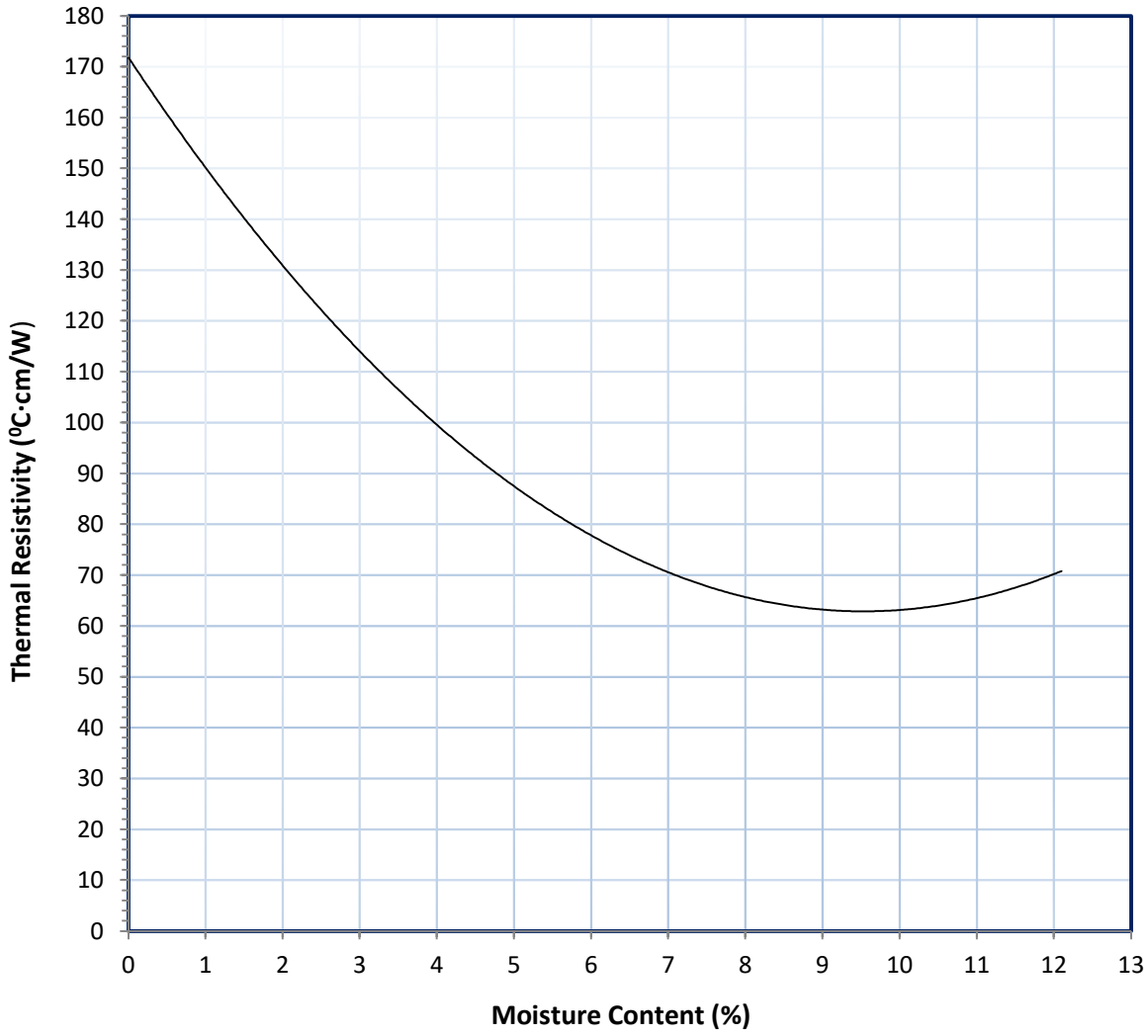


Specimen ID:	B-9 to B-14, Bulk 3, 0.0-5.0 ft
USCS:	CL-ML
Received Moisture:	12.0%
LL:	20
PI:	6
P200:	53.4%
Max. Dry Dens.:	119.0 pcf
Optimum Moisture:	11.9%

Specimen was prepared at the optimum moisture content and at approximately 90% of the maximum dry density as determined by the Standard Proctor test.

B-22 to B-29, Bulk 4, 0.0-5.0 ft

THERMAL RESISTIVITY DRY-OUT CURVES
411360.GEO1: Mill Point Solar Project

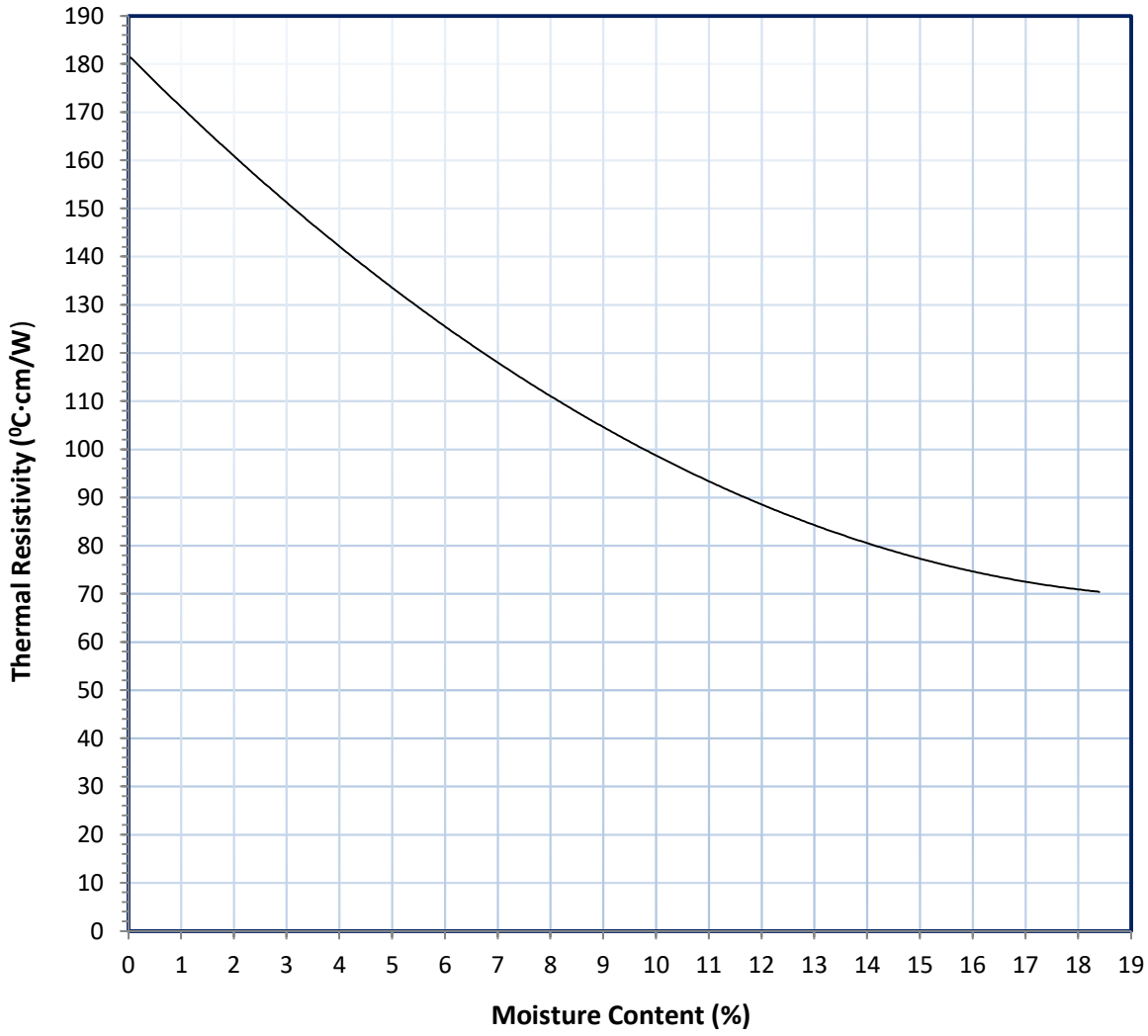


Specimen ID:	B-22 to B-29, Bulk 4, 0.0-5.0 ft
USCS:	CL-ML
Received Moisture:	19.7%
LL:	18
PI:	4
P200:	76.5%
Max. Dry Dens.:	114.5 pcf
Optimum Moisture:	12.1%

Specimen was prepared at the optimum moisture content and at approximately 90% of the maximum dry density as determined by the Standard Proctor test.

SS-1 & SS-2, Bulk 5, 0.0-5.0 ft

THERMAL RESISTIVITY DRY-OUT CURVES
411360.GEO1: Mill Point Solar Project



Specimen ID:	SS-1 & SS-2, Bulk 5, 0.0-5.0 ft
USCS:	CL
Received Moisture:	14.5%
LL:	43
PI:	18
P200:	73.7%
Max. Dry Dens.:	96.2 pcf
Optimum Moisture:	18.4%

Specimen was prepared at the optimum moisture content and at approximately 90% of the maximum dry density as determined by the Standard Proctor test.

KE CORROSION

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702-340-1186 KDE@KECORROSION.COM

CLIENT
TRC Solutions, Inc.
1600 Commerce Parkway, Suite B
Mount Laurel, NJ 08054

PROJECT NO: 411360

PROJECT
Mill Point

DATE: June 1, 2021

LAB ID: 21-0068

Sample By: Client

Analyzed By: Kurt D. Ergun

RESULTS FOR CORROSIVITY ANALYSIS OF SOILS

Sample No:	Bulk
Sample Location:	SS-1 to SS-2
Sample Depth:	0.0-5.0
Laboratory Testing Methods	
pH Analysis, ASTM D4972(in H20)	8.34
pH Analysis, ASTM D4972(in CaCl2)	8.00
Water Solube Sulfates, ASTM D516(mg/kg)	185
Clorides, ASTM D512(mg/kg)	75
Resistivity, ASTM G57(ohm-cm)	1568

Kurt D. Ergun
Chemist

Note: The tests were performed in accordance with applicable ASTM, AASHTO, or AWWA methods. Test results submitted are only applicable to samples tested at referenced locations and are not indicative of the results of similar materials.

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RESULTS FOR CORROSIVITY ANALYSIS OF SOILS

Sample No:	Bulk	Bulk	Bulk	Bulk
Sample Location:	B-1 to B-5	B-7 to B-8	B-9 to B-14	B-22 to B-29
Sample Depth:	0.0-5.0	0.0-5.0	0.0-5.0	0.0-5.0
Laboratory Testing Methods				
pH Analysis, ASTM D4972(in H20)	7.71	8.08	8.40	8.21
pH Analysis, ASTM D4972(in CaCl2)	7.36	7.45	7.45	8.03
Water Solube Sulfates, ASTM D516(mg/kg)	58	55	220	235
Clorides, ASTM D512(mg/kg)	50	40	40	38
Resistivity, ASTM G57(ohm-cm)	2548	2940	1260	1176

Kurt D. Ergun
Chemist

Note: The tests were performed in accordance with applicable ASTM, AASHTO, or AWWA methods. Test results submitted are only applicable to samples tested at referenced locations and are not indicative of the results of similar materials.